

# Beggs Deformeter Stress Analysis of Single-Barrel Conduits 

UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF RECLAMATION

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Bureau programs most frequently are the result of close cooperation with the U.S. Congress, other Federal agencies, States, local governments, academic institutions, water-user organizations, and other concerned groups.

# Beggs Deformeter Stress Analysis of Single-Barrel Conduits 

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#### Abstract

As the Nation's principal conservation agency, the Department of the Interior has responsibility for most of our nationally owned public lands and natural resources. This includes fostering the wisest use of our land and water resources, protecting our fish and wildilife, preserving the environmental and cultural values of our national parks and historical places, and providing for the enjoyment of life through outdoor recreation. The Department assesses our energy and mineral resources and works to assure that their development is in the best interests of all our people. The Department also has a major responsibility for American Indian reservation communities and for people who live in Island Territories under U.S. Administration.


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This monograph presents the results of the stress analysis, by means of the Beggs Deformeter apparatus, ${ }^{1}$ of nine shapes of single-barrel conduits. A partial analytical oheck was made using the least work method to determine the redundant reactions for all shapes due to a uniform vertical load and a uniform horizontal load.

All personnel of the Experimental Design Analysis Section, including several rotation engineers who had training assignments in the section, assisted in the experimental work and computations. In particular, the assistance of W. T. Moody in computing the analytical solutions, and the work of H. E. Willmann, who prepared the drawings and also assisted in the experimental work and computations, is gratefully acknowledged.

The nine shapes of conduits studied are those most widely used in Bureau of Reclamation structures. All except shape $D$ and the square shape have semicircular top portions of uniform thickness. They can be further described as follows:

1. Shape A: horseshoe-shaped interior with a horizontal exterior base.
2. Shape B: circular-shaped interior with a horizontal exterior base.
3. Shape C: circular-shaped interior with a curved exterior base.
4. Shape D: circular-shaped interior with a square-shaped exterior.
5. Shape E: uniform thickness with a horizontal base.
6. Shape F: uniform thickness of horseshoe shape.
7. Shape G: transition between shape $B$ and shape E with fillets of $1 / 2 r$ radius in lower interior corners.
8. Circular shape of uniform thickness.
9. Square shape of uniform thickness.

Reaction coefficients for bending moment, thrust, and shear at selected locations along the centroidal axis of the conduits have been determined for 15 different loading conditions.
${ }^{1}$ See Appendix for description of this instrument.

The 15 loading conditions considered are as follows:

1. $I$ top with 4 foundation.
2. 7 top with foundation.
3. I top with $\square \mp+1$ foundation.
4. top with $\square$ foundation.
5. $A D$ top with $\square+1$ foundation.
6. $\Delta$
7. $\square$ top with $\square+\ddagger$ foundation.
8. $\square$ top with foundation.
9. Dead load with foundation.
10. Uniform horizontal both sides.
11. $\Delta$ horizontal both sides.
12. Uniform internal radial.
13. $\Delta$ internal radial with $4 \mathbb{1}$ foundation.
14. $\Delta$ internal radial with $\prod$ foundation.
15. $\Delta$ external hydrostatic including dead load.

Figures 1, 2, and 3 show cross sections of each shape, giving the dimensions and the location of points at which the reaction coefficients have been determined.
Each shape was analyzed for three values of crown thickness, $t$, expressed in terms of the internal crown radius, $r$. These three values were $t=r / 2, t=r / 3$, and $t=r / 6$. A conduit of unit length was considered in the analysis. Bending moment, thrust, and shear coefficients were determined at the various locations shown, and are expressed in terms of unit intensity of loading and unit internal crown radius. Multiplying the reaction coefficient by the proper load factor gives the total bending moment, thrust, or shear at the centroid of the section under consideration.

## APPLICATION

The reaction coefficients determined in the study are tabulated in figures 4 through 50 for the various shapes and loading conditions. The reaction coefficients are given for points on the right side of the conduits only, since the conduits and loadings are symmetrical about the vertical centerline. The shear reactions on the left side of the vertical centerline will have an opposite sign from those given for the points on the right side.

Consistent units should be employed when using these data. Thus, if loads are expressed in pounds per square inch, all dimensions of the conduit must be expressed in inches. The bending moment will then be in inch-pounds per inch of conduit length and the thrust and shear in pounds per inch of conduit length. If the load is expressed in terms of pounds per square foot, the dimensions of the conduit must be expressed in feet, and the bending moment will be in foot-pounds per foot of conduit length and the thrust and shear in pounds per foot of conduit length. It will be noted that the bending moment in inch-pounds per inch is numerically equal to the bending moment in foot-pounds per foot.

One should bear in mind that this analysis assumes no restraint to the deformation of the conduit.

In some cases this restraint, or passive pressure, may be important. Some work on passive pressures on tunnel linings through rock has been done by R. S. Sandhu. ${ }^{2}$ By using his method for determining the intensity of the passive pressure, and using the moment, thrust, and shear coefficients

[^0]for a circular conduit given by figure 50 , the effect of restraint may be approximated.

The foundation load distribution due to a vertical load on the conduit must be assumed, and is influenced by the modulus of elasticity of the foundation material. As the foundation modulus increases, the foundation load distribution approaches a concentration at the outside corners of the conduit, and as it decreases the load approaches a uniform distribution. For all vertical loading conditions except three, two distributions were assumed, viz., uniform, and triangular with zero at the center and maximum at the outside corners.

For the dead load the assumed foundation reaction is minimum at the center varying linearly to a maximum at the outside corners, with the intensity at the center equal to the intensity of the weight of the conduit at the center of the base.

For the triangular internal radial load the assumed foundation reactions were uniform, and triangular with zero at the outsides and maximum at the center.

For the triangular external hydrostatic load; including dead load, the unit weight of the conduit material and the unit weight of water were assumed to be 150 and 62.4 pounds per cubic foot, respectively. With these assumptions the weight of the conduit for the $t=r / 6$ case, except shape D , is less than the uplift, causing the conduit to float. The reaction is assumed to be uniformly distributed across the top. The coefficients for this assumption (conduit floating) are given in figure 49. In the other figures of this loading condition, tension is assumed to develop uniformly along the foundation.

## DETERMINATION OF NORMAL STRESS DISTRIBUTION

In a curved beam the neutral axis will not be coincident with the centroidal axis, and the normal stress distribution on radial lines, due to moment, will not be linear. However, the radius to the neutral axis and the normal stress distribution may be determined by the following equations, derived from the Winkler-Bach theory for curved beams: ${ }^{3}$


$$
\begin{equation*}
r_{n}=\frac{t}{\ln \left(r_{o} / r\right)} \tag{1}
\end{equation*}
$$

where
$r_{n}$ is the radius to the neutral axis
$r$ is the internal radius
$r_{o}$ is the external radius
$t$ is the wall thickness ( $r_{o}-r$ )
$\ln$ is the $\log$ to the base $e$,

$$
\begin{equation*}
\sigma_{\theta}=\frac{T}{t}+\frac{M y_{n}}{\left(r_{n}+y_{n}\right) t e} \tag{2}
\end{equation*}
$$

where
$\sigma_{\theta}$ is the normal stress in the tangential direction
$M$ is the bending moment at the centroidal axis

[^1]$T$ is the thrust at the centroidal axis
$y_{n}$ is the distance from the neutral axis to the point of interest (positive outward)
$e$ is the distance from the centroidal axis to the neutral axis.

As $t$ decreases $e$ approaches zero, and the $\sigma_{\theta}$ distribution approaches linearity.
$\sigma_{\theta}$, as computed by equation (2), is only for a constant thickness section. Where the section thickness is not constant, the distribution of stresses must be determined by some other method, such as photoelasticity.

The extreme fiber stress in a constant thickness curved beam due to bending moment may be determined by the equation:

$$
\begin{equation*}
\sigma_{b}=K \frac{M t}{2 I} \tag{3}
\end{equation*}
$$

where
$\sigma_{b}$ is the extreme fiber stress
$M$ is the bending moment at the section
$t$ is the width of the section
$I$ is the moment of inertia of the section
$K$ is the factor by which the extreme fiber stress, assuming linear distribution, is modified to correct for curvature.
The following equation for $K$ was obtained by equating equations (2) and (3):

$$
\begin{equation*}
K=\frac{t y_{n}}{6\left(r_{n}+y_{n}\right) e} \tag{4}
\end{equation*}
$$

The values of $K$ and $e$ for the $t / r$ ratios used in this study are tabulated below:

Table 1.-Correction factors for different radii of curvature

|  | $K$ |  |  |
| :---: | :---: | :---: | :---: |
| $t$ | Inside <br> fiber | Outside <br> fiber |  |
| $r / 2$ | 1.153 | 0.880 | $0.0168 r$ |
| $r / 3$ | 1.105 | 0.912 | $0.0080 r$ |
| $r / 6$ | 1.054 | 0.951 | $0.0021 r$ |






$t=\frac{r}{2} \quad t=\frac{r}{3} \quad t=\frac{r}{6}$

| POINT | $\frac{M}{v r^{2}}$ | $\frac{T}{v r}$ | $\frac{S}{v r}$ | $\frac{M}{v r^{2}}$ | $\frac{T}{v r}$ | $\frac{S}{v r}$ | $\frac{M}{v r^{2}}$ | $\frac{T}{v r}$ | $\frac{S}{v r}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +0.302 | +0.132 | 0 | +0.280 | +0.113 | 0 | +0.244 | +0.102 | 0 |
| 2 | +0.257 | +0.228 | +0.341 | +0.240 | +0.198 | +0.304 | +0.209 | +0.177 | +0.265 |
| 3 | +0.136 | +0.489 | +0.583 | +0.131 | +0.431 | +0.521 | +0.113 | +0.380 | +0.454 |
| 4 | -0.025 | +0.843 | +0.657 | -0.015 | +0.746 | +0.587 | -0.015 | +0.656 | +0.511 |
| 5 | -0.178 | +1.191 | +0.535 | -0.155 | +1.056 | +0.480 | -0.138 | +0.926 | +0.417 |
| 6 | -0.276 | +1.434 | +0.247 | -0.245 | +1.273 | +0.224 | -0.218 | +1.115 | +0.193 |
| 7 | -0.283 | +1.500 | -0.132 | -0.255 | +1.333 | -0.113 | -0.228 | +1.167 | -0.102 |
| 8 | -0.229 | +1.488 | -0.231 | -0.209 | +1.323 | -0.202 | -0.187 | +1.157 | -0.182 |
| 9 | -0.145 | +1.470 | -0.329 | -0.137 | +1.306 | -0.290 | -0.124 | +1.142 | -0.261 |
| 10 | -0.033 | +1.445 | -0.425 | -0.039 | +1.284 | -0.378 | -0.037 | +1.121 | -0.338 |
| 11 | +0.091 | -0.009 | -0.627 | +0.071 | +0.019 | -0.654 | +0.050 | +0.040 | -0.692 |
| 12 | +0.205 | -0.096 | -0.283 | +0.192 | -0.074 | -0.295 | +0.181 | -0.060 | -0.311 |
| 13 | +0.250 | -0.127 | -0.075 | +0.239 | -0.108 | -0.077 | +0.231 | -0.097 | -0.081 |
| 14 | +0.257 | -0.132 | 0 | +0.246 | -0.113 | 0 | +0.239 | -0.102 | 0 |



| Point | $\frac{M}{v r^{2}}$ | $\frac{\mathrm{I}}{\mathrm{v}}$ | $\frac{5}{v r}$ | $\frac{M}{v r^{2}}$ | $\frac{\mathrm{T}}{\mathrm{v}}$ | $\frac{5}{v r}$ | $\frac{M}{v r^{2}}$ | $\frac{T}{\mathrm{v}}$ | $\frac{s}{v r}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +0.303 | +0.135 | 0 | +0.274 | +0.119 | 0 | -0.23 | +0.116 | 0 |
| 2 | +0.259 | +0.231 | +0.340 | +0.234 | +0.204 | +0.302 | +0.200 | +0.191 | $+0.262$ |
| 3 | +0.138 | +0.492 | r0.582 | +0.126 | +0.437 | +0.518 | +0.106 | +0.392 | $+0.4$ |
| 4 | -0.022 | +0.845 | +0.6.55 | -0.019 | +0.751 | +0.582 | 20 | +0.666 | +0.5 |
| 5 | -0.175 | +1.192 | $+0.533$ | -0.157 | +1.060 | +0.474 | -0.140 | +0.933 | +0.404 |
| 6 | -0.271 | +1.434 | +0.243 | -0.245 | + 1.275 | +0.218 | -0.216 | 19 | +0.179 |
| 7 | -0.278 | +1.500 | -0.135 | -0.254 | +1.333 | -0.119 | -0.223 | +1.167 | -0.116 |
| 8 | -0.215 | +1.475 | -0.305 | -0.198 | + 1.310 | -0.275 | -0.172 | +1.144 | -0.257 |
| 9 | -0.103 | $+1.431$ | -0.470 | -0.100. | +1.269 | -0.426 | -0.084 | $+1.104$ | -0394 |
| 10 | +0.056 | $+1.368$ | -0.630 | +0.040 | + 1.210 | -0.572 | +0.040 | +1.049 | -0.525 |
| 11 | +0. 123 | +0.092 | -0.660 | +0.108 | +0.116 | -0.660 | +0.105 | +0.129 | -0.665 |
| 12 | +0.217 | -0.069 | -0.301 | +a207 | -0.051 | -0.299 | -0.209 | -0.045 | -0.301 |
| 13 | +0.258 | -0.126 | -0.082 | +0.248 | -0.110 | -0.081 | +0.252 | -0.107 | . 082 |
| 14 | +0.265 | -0.135 | 0 | +0.256 | -0.119 | 0 | +0.260 | -0.116 | 0 |



| POINT | $\frac{M}{v r^{2}}$ | $\frac{T}{v r}$ | $\frac{5}{\mathrm{vr}}$ | $\frac{\mathrm{M}}{\mathrm{Vr}}{ }^{2}$ | $\frac{\mathrm{T}}{\mathrm{vr}}$ | $\frac{\mathrm{S}}{\mathrm{vr}}$ | $\frac{\mathrm{M}}{\mathrm{vr}}{ }^{2}$ | $\frac{\mathrm{T}}{\mathrm{va}}$ | $\frac{s}{v r}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +0.309 | $+0.109$ | 0 | +0.287 | $+0.085$ | 0 | +0.253 | +0.065 | 0 |
| 2 | $+0.263$ | $+0.205$ | $+0.347$ | +0.245 | +0.172 | $+0.311$ | $+0.217$ | $+0.141$ | $+0.275$ |
| 3 | $+0.140$ | +0.469 | +0.595 | +0.133 | $+0.407$ | +0.535 | $+0.117$ | $+0.348$ | $+0.473$ |
| 4 | -0.026 | +0.827 | $+0.673$ | -0.018 | $+0.727$ | $+0.606$ | -0.018 | $+0.629$ | +0.538 |
| 5 | -0.186 | +1.179 | +0.556 | -0.164 | $+1.043$ | +0.504 | -0.149 | $+0.907$ | +0.449 |
| 6 | -0.290 | +1.428 | +0.270 | -0.262 | $+1.266$ | + 0.251 | -0.239 | $+1.105$ | +0.229 |
| 7 | -0.305 | $+1.500$ | -0.109 | -0.281 | $+1.333$ | -0.085 | -0.260 | $+1.167$ | -0.065 |
| 8 | -0.244 | $+1.476$ | -0.291 | -0.230 | $+1.312$ | -0.253 | -0.219 | $+1.148$ | -0.216 |
| 9 | -0.128 | $+1.429$ | -0.469 | -0.131 | $+1.270$ | -0.416 | -0.136 | $+1.110$ | -0.364 |
| 10 | -0.078 | +0.635 | -0.908 | -0.088 | +0.619 | -0.841 | -0.103 | $\div 0.599$ | -0.774 |
| 11 | +0.064 | +0.259 | -0.587 | +0.055 | +0.254 | -0.533 | $+0.040$ | +0.246 | -0.479 |
| 12 | +0.157 | +0.037 | -0.317 | +0.146 | +0.043 | -0.277 | +0.128 | +0.046 | -0.238 |
| 13 | +0.197 | -0.087 | -0.102 | +0.182 | -0.067 | -0.088 | +0.161 | -0.049 | -0.075 |
| 14 | +0.205 | -0.109 | 0 | +0.189 | -0.085 | 0 | +0.167 | -0.065 | 0 |



## SINGLE BARREL CONDUIT

 BEGGS DEFORMETER STRESS ANALYSIS COEFFIGIENTS FOR MOMENT, THRUST, AND SHEAR UNIFORM VERTIGAL LOAD-TRIANGULAR FOUNDATION REACTION
$t=\frac{r}{3}$
$t=\frac{r}{6}$

| POINT | $\frac{M}{v r^{2}}$ | $\frac{7}{v r}$ | $\frac{s}{\text { sr }}$ | $\frac{\mathrm{M}}{\mathrm{r}^{2}}$ | $\frac{T}{v r}$ | $\frac{s}{v r}$ | $\frac{M}{v r^{2}}$ | $\frac{\mathrm{T}}{\mathrm{v}}$ | $\frac{s}{\text { sr }}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +1.062 | +0.240 | +1.500 | +0.873 | 0.214 | +1.333 | +0.712 | 184 |  |
| 2 | +0.587 | +0.621 | +1.387 | +0.479 | 0.5 | +1.23 | +0.392 | +0.480 |  |
| 3 | +0.165 | +0.958 | +1.179 | +0.128 | +0.85 | +1.04 | +0.107 | +0.7 | +0 |
| 4 | -0.176 | +1.23 | +0.891 | -0.154 | +1.094 | +0.79 | -0.123 | +0.955 | $+0.695$ |
| 5 | -0.411 | +1.419 | +0.542 | -0.349 | +1.262 | $+0.48$ | -0. | +1.10 | +0.424 |
| 6 | -0.526 | +1.511 | -0.156 | -0.444 | +1.343 | +0.138 | -0.36 | +1. | +0.124 |
| 7 | -0.512 | +1.500 | -0.240 | -0.433 | +1.333 | -0.214 | -0.352 | +1.167 |  |
| 8 | -0.426 | +1.481 | -0.339 | -0.357 | +1,316 | $-0.303$ | -0.288 | +1.151 | -0 |
| 9 | -0.310 | +1.455 | -0.436 | -0.255 | +1.293 | -0.391 | -0.201 | +1.131 | -0.342 |
| 10 | -0.165 | +1.423 | -0.531 | -0.127 | +1.263 | -0.477 | -0.091 | +1.105 | -0.418 |
| 11 | +0.123 | -0.047 | -0.987 | +0.083 | -0.024 | -0.95 | +0.046 | . 03 | 913 |
| 12 | +0.352 | -0.155 | -0.661 | +0.304 | -0.130 | -0.63 | +0.258 | 0.10 | -0.611 |
| 13 | +0.491 | -0.219 | -0.331 | +0.439 | -0.193 | 0.31 | +0.387 | -0.163 | -0.306 |
| 14 | +0.538 | -0.240 | 0 | $+0.484$ | . 214 | 0 | +0.430 | -0.184 | 0 |


| point | $\frac{M}{v r^{2}}$ | $\frac{T}{v r}$ | $\frac{s}{v r}$ | $\frac{M}{\mathrm{v} \mathrm{r}^{2}}$ | $\frac{T}{\text { vr }}$ | $\frac{5}{v r}$ | $\frac{\mathrm{m}}{\mathrm{c} \mathrm{r}^{2}}$ | $\frac{T}{\text { vr }}$ | $\frac{5}{v r}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | $\cdots$ | +0.255 | +1.500 | +0.866 | +0.221 | *1.333 | +0.700 | +0.206 | +1.167 |
| 2 | -0.569 | +0.634 | +1.383 | +0.472 | +0.559 | +1.231 | +0.380 | +0.501 | +1.07 |
| 3 | +0.148 | +0.971 | -1.172 | $+0.122$ | +0.858 | +1.044 | +0.098 | +0.762 | +0.907 |
| 4 | -0.189 | +1.241 | +0.881 | 0.159 | +1.09 | +0.78 | -0.129 | +0.971 | +0.679 |
| 5 | 421 | -1.426 | +0.529 | -0.353 | +1.265 | $+0.475$ | -0.283 | +1.113 | +0.405 |
| 6 | -0.532 | +1.515 | $+0.142$ | -0.446 | +1.345 | $+0.132$ | -0.356 | +1 | -0.103 |
| 7 | -0.513 | +1.500 | -0.255 | -0.432 | +1.333 | -0.221 | -0.341 | $+1.16$ | -0.206 |
| 8 | -0.415 | $\cdot 1.461$ | -0.424 | -0.349 | +1.298 | -0.376 | -0.266 | -1.133 | -0.346 |
| 9 | -0.270 | $\cdot 1.404$ | -0.587 | 0.222 | +1.245 | -0.525 | -0.154 | +1.083 | -0.481 |
| 10 | -0.078 | +1.328 | -0.743 | -0.055 | +1.175 | -0.668 | 0.006 | +1.017 | -0.608 |
| 11 | +0.168 | +0.092 | -1.017 | +0.128 | +0.116 | -0.956 | +0.123 | +0.122 | -0.898 |
| 12 | +0.378 | -0.102 | -0.686 | +0.326 | -0.073 | -0.645 | +0.309 | -0.062 | 0.606 |
| 13 | +0.510 | -0.217 | -0.345 | +0.450 | -0.184 | -0.325 | +0.426 | -0.170 | 0.305 |
| 14 | +0.555 | -0.255 | 0 | +0.492 | -0.221 | 0 | +0.466 | -0.206 | 0 |



| POINT | $\frac{M}{v r^{2}}$ | $\frac{1}{v r}$ | $\frac{5}{v r}$ | $\frac{M}{v r^{2}}$ | $\frac{\mathrm{T}}{\mathrm{v}}$ | $\frac{s}{v r}$ | $\frac{M}{v r^{2}}$ | $\frac{T}{\mathrm{v}}$ | $\frac{s}{v r}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | . 051 | +0.21 | $-1.500$ | +0.88 | +0.172 | +1.333 | +0.724 | -0.127 | 167 |
| 2 | +0.575 | +0.593 | +1.394 | +0.481 | +0.511 | +1.244 | +0.402 | +0.425 | $+1.094$ |
| 3 | +0.149 | +0.934 | $+1.193$ | +0.13 | -0.815 | 1.06 | $+0.11$ | +0.694 | +0.947 |
| 4 | 0.197 | +1.211 | +0.911 | -0.157 | +1.064 | +0.822 | -0.12 | +0.915 | +0.73 |
| 5 | -0.440 | +1.405 | +0.566 | -0.363 | +1.240 | +0.518 | -0.30 | +1.074 | +0.473 |
| 6 | -0.564 | +1.504 | +0.183 | 0.470 | 1.332 | 0.1 | -0.394 | +1.160 | +0.17 |
| 7 | 59 | +1.500 | 0.212 | 0.472 | -1.333 | -0.172 | -0.402 | +1.16 | 127 |
| 8 | -0.466 | +1.463 | 0.39 | -0.396 | +1.30 | -0.33 | -0.343 | +1.1 | -0.278 |
| 9 | -0.318 | +1.404 | 0.56 | -0.271 | +1.248 | -0.49 | -0.242 | +1.00 | -0.425 |
| 10 | -0.191 | +0.676 | 1.115 | -0.183 | +0.644 | -1.00 | -0.19 | +0.6 | -0.884 |
| 11 | +0.067 | +0.362 | -0.929 | +0.051 | +0.348 | -0.829 | +0020 | +0.3 | -0.726 |
| 12 | +0.278 | +0.110 | -0.695 | +0.241 | +0.113 | -0.610 | +0.188 | +0.119 | -0.524 |
| 13 | 09 | -0.127 | -0.384 | +0.355 | -0.097 | 0.338 | +0.285 | 0.062 | -0.290 |
| 14 | +0.456 | -0.212 | 0 | +0.395 | -0.172 | 0 | +0.320 | -0.127 | 0 |



SINGLE BARREL CONDUIT BEGGS DEFORMETER STRESS ANALYSIS COEFFICIENTS FOR MOMENT, THRUST, AND SHEAR congentrated vertical load - uniform foundation reaction shapes a, b, and C

[^2]

| POINT. | $\frac{M}{v r^{2}}$ | $\frac{T}{v r}$ | $\frac{s}{v r}$ | $\frac{M}{v r^{2}}$ | - $\frac{T}{r}$ | $\frac{S}{v r}$ | $\frac{\mathrm{M}}{\mathrm{v} \mathrm{r}^{2}}$ | $\frac{\mathrm{T}}{\mathrm{vr}}$ | $\frac{s}{v r}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +1.007 | +0.340 | +1.500 | t0.835 | +0.297 | +1.333 | +0.683 | +0.256 | +1.167 |
| 2 | +0.536 | $+0.716$ | $+1.361$ | +0.444 | +0.632 | $+1.211$ | +0.365 | +0.549 | +1.061 |
| 3 | +0.126 | $+1.044$ | +1.129 | +0.104 | +0.924 | +1.006 | +0.088 | +0.805 | +0.883 |
| 4 | -0.194 | +1.301 | +0.820 | -0.163 | $+1.153$ | +0.733 | -0.130 | +1.006 | +0644 |
| 5 | -0.404 | +1.469 | +0.456 | -0.339 | +1.303 | +0.410 | -0.274 | +1.138 | +0362 |
| 6 | -0.489 | $+1.537$ | +0.060 | -0.411 | +1.365 | \$0059 | -0.333 | +1.193 | +0055 |
| 7 | -0.443 | +1.500 | -0.340 | -0.374 | +1.333 | -0.297 | -0.305 | +1.167 | -0.256 |
| 8 | -0.327 | +1.474 | -0.438 | -0.274 | $+1.310$ | -0.386 | -0.219 | $+1.146$ | -0.335 |
| 9 | -0.182 | $+1.442$ | -0.534 | -0.148 | +1.282 | -0.473 | -0.112 | +1.121 | -0.413 |
| 10 | -0.008 | + 1.404 | -0.628 | +0.003 | +1.247 | -0.558 | +0.018 | $+1.090$ | -0.488 |
| 11 | +0.172 | -0.212 | -0.668 | +0.140 | -0.161 | -0.691 | +0.126 | -0.110 | -0.724 |
| 12 | +0.297 | -0.301 | -0.311 | +0.271 | -0.256 | -0.319 | +0.265 | -0.212 | -0.332 |
| 13 | +0.347 | -0.335 | -0.089 | +0.333 | -0.291 | -0.090 | +0.319 | -0.250 | -0092 |
| 14 | +0.357 | -0.340 | 0 | +0.342 | -0.297 | 0 | +0.329 | -0.256 | 0 |



| POINT | $\frac{\mathrm{M}}{v r^{2}}$ | $\frac{\mathrm{~T}}{\mathrm{vr}}$ | $\frac{\mathrm{S}}{\mathrm{vr}}$ |
| :---: | :---: | :---: | :---: |
| 1 | +0.994 | +0.362 | +1.500 |
| 2 | +0.524 | +0.738 | +1.355 |
| 3 | +0.117 | +1.063 | +1.118 |
| 4 | -0.200 | +1.316 | +0.805 |
| 5 | -0.404 | +1.480 | +0.437 |
| 6 | -0.482 | +1.542 | +0.039 |
| 7 | -0.429 | +1.500 | -0.362 |
| 8 | -0.301 | +1.449 | -0.530 |
| 9 | -0.124 | +1.379 | -0.691 |
| 10 | +0.097 | +1.292 | -0.844 |
| 11 | +0.222 | -0.122 | -0.736 |
| 12 | +0.335 | -0.290 | -0.352 |
| 13 | +0.387 | -0.352 | -0.108 |
| 14 | +0.398 | -0.362 | 0 |


| $\frac{M}{v r^{2}}$ | $\frac{T}{v r}$ | $\frac{S}{v r}$ |
| :---: | :---: | :---: |
| +0.829 | +0.311 | +1.333 |
| +0.439 | +0.645 | +1.208 |
| +0.100 | +0.936 | +0.999 |
| -0.165 | +1.162 | +0.723 |
| -0.337 | +1.310 | +0.398 |
| -0.405 | +1.368 | +0.045 |
| -0.364 | +1.333 | -0.311 |
| -0.255 | +1.288 | -0.465 |
| -0.104 | +1.224 | -0.612 |
| +0.087 | +1.144 | -0.752 |
| +0.193 | -0.064 | -0.726 |
| +0.307 | -0.237 | -0.344 |
| +0.358 | -0.300 | -0.104 |
| +0.369 | -0.311 | 0 |


| $\frac{M}{r^{2}}$ | $\frac{T}{v r}$ | $\frac{5}{\mathrm{vr}}$ |
| :---: | :---: | :---: |
| +0.669 | +0.284 | +1.167 |
| +0.353 | +0.576 | $+1.0$ |
| +0.079 | +0.829 | +0.868 |
| -0.134 | 1.026 | +0.624 |
| -0.271 | +1.152 | +0. |
| 323 | +1.200 | +0.028 |
| -0.287 | 1.67 | -0.284 |
| -0.191 | +1.12 | -0.423 |
| -0.058 | +1.06 | -0.55 |
| +0.110 | +0.989 |  |
| +0.198 | -0.028 | -0.725 |
| +0.315 | -0.208 | -0.342 |
| +0.367 | -0.274 | 0.10 |
| +0.378 | $-0.284$ | $\bigcirc$ |



| Point | $\frac{M}{\mathrm{v}^{2}}$ | $\frac{\mathrm{T}}{\mathrm{v}}$ | $\frac{5}{\mathrm{v}}$ | $\frac{M}{v r^{2}}$ | $\frac{T}{\mathrm{v}}$ | $\frac{5}{\mathrm{wr}}$ | $\frac{\mathrm{M}}{\mathrm{v} \mathrm{r}^{2}}$ | T T | $\frac{s}{\mathrm{v}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | +0.992 | +0.324 | +1.500 | +0.843 | ro. 265 | +1.333 | +0.692 | 10.208 | $+1.167$ |
| 2 | +0.520 | +0.701 | +1.365 | +0.451 | +0.601 | +1.219 | +0.372 | +0.502 | +1.073 |
| 3 | +0. 108 | +1.031 | +1.13? | +0. 106 | +0.896 | +1.022 | +0.090 | +0.763 | +0.907 |
| 4 | -0.216 | +1.290 | +0.831 | -0.167 | +1.130 | +0.75 | 0.136 | +0.972 | +0.678 |
| 5 | -0.430 | +1.461 | +0.469 | -0.350 | +1.287 | +0.437 | -0.290 | +1.114 | +0.404 |
| 6 | -0.519 | +1.533 | +0.075 | -0.431 | +1.356 | +0.089 | -0.363 | +1.181 | +0.10 |
| 7 | -0.478 | +1.500 | -0.324 | -0.404 | +1.333 | -0.26 | -0.34 | $+1$. | -0.208 |
| 8 | -0.351 | +1.449 | -0.505 | -0.300 | +1.289 | -0.43 | -0.266 | +1.130 | 0.356 |
| 9 | -0.169 | +1.377 | -0.678 | -0.14 | +1.225 | -0.590 | -0.142 | +1.073 | -0.502 |
| 10 | -0.083 | +0.471 | $-1.048$ | -0.085 | +0.484 | -0.959 | -0.098 | +0.493 | -0.069 |
| 11 | +0.099 | +0.080 | -0.706 | +0.092 | +0. 105 | -0.633 | +0.070 | +0.128 | -0.559 |
| 12 | +0.226 | -0.156 | -0.413 | +0.210 | -0.118 | -0.357 | +0.178 | -0.081 | -0.302 |
| 13 | +0.287 | 0.297 | -0.152 | +0.263 | -0.242 | -0.129 | +0.224 | -0.188 | -0.108 |
| 14 | +0.302 | -0.324 | 0 | +0.275 | -0.265 | 0 | +0.234 | -0.208 | 0 |





|  |  | $t=\frac{r}{2}$ |  |  |  | $t=\frac{r}{3}$ |  |  | $t=\frac{1}{6}$ |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & 4 \\ & \mathbf{a} \\ & \frac{1}{4} \\ & \frac{1}{\infty} \end{aligned}$ |  | POINT | $\frac{M}{9 r^{3}}$ | $\frac{1}{9 r^{2}}$ | $\frac{5}{9 r^{2}}$ | $\frac{\mathrm{M}}{9 \mathrm{r}^{3}}$ | $\frac{T}{9 r^{2}}$ | $\frac{5}{g r^{2}}$ | $\frac{M}{\text { gr }}$ | $\frac{\mathrm{T}}{\mathrm{gr}}{ }^{2}$ | $\frac{5}{9 r t}$ |
|  |  | 1 | +0.028 | -0.022 | 0 | +0.030 | -0.015 | 0 | +0.024 | -0.009 | 0 |
|  |  | 2 | +0.027 | -0.020 | +0.012 | +0.029 | -0.014 | +0.009 | +0.024 | -0.008 | +0.006 |
|  |  | 3 | +0.021 | +0.005 | +0.053 | +0.024 | +0.006 | +0.041 | +0.020 | +0.007 | +0.030 |
|  |  | 4 | +0.008 | +0.087 | +0.118 | +0.012 | +0.070 | +0.092 | +0.009 | +0.056 | +0.068 |
|  |  | 5 | -0.011 | +0.234 | +0.161 | -0.008 | +0.186 | +0.125 | -0.009 | +0.144 | +0.094 |
|  |  | 6 | -0.030 | +0.399 | $+0.130$ | -0.028 | +0.316 | +0.101 | -0.028 | +0.243 | +0.074 |
|  |  | 7 | -0.041 | +0.483 | +0.022 | -0.038 | +0.382 | +0.015 | -0.037 | +0.292 | +0.009 |
|  |  | 8 | -0.043 | +0.483 | -0.010 | -0.039 | +0.382 | -0.010 | -0.037 | +0.292 | -0.011 |
|  |  | 9 | -0.035 | +0.482 | -0.04.2 | -0.032 | +0.380 | -0.036 | -0.03i | +0.291 | -0.031 |
|  |  | 10 | -0.018 | +0.478 | -0.074 | -0.018 | +0.377 | -0.061 | -0.019 | +0.288 | -0.051 |
|  |  | 11. | $+0.005$ | +0.060 | -0.189 | +0.003 | +0.052 | -0.178 | -0.002 | +0.044 | -0.166 |
|  |  | 12 | +0.038 | +0.033 | -0.083 | +0.036 | +0.026 | -0.078 | +0.029 | +0.019 | -0.073 |
|  |  | 13 | +0.051 | +0.023 | -0.020 | +0.048 | +0.017 | -0.019 | +0.040 | +0.010 | -0.018 |
|  |  | 14 | +0.052 | +0.022 | 0 | +0.050 | +0.015 | 0 | +0.042 | +0.009 | 0 |


|  |  | POINT | $\frac{\mathrm{M}}{\mathrm{g}} \mathrm{m}^{\text {3 }}$ | $\frac{\mathrm{T}}{\mathrm{gra}}$ | $\frac{\mathrm{S}}{\mathrm{gr}}{ }^{\text {2 }}$ | $\frac{\mathrm{M}}{\mathrm{gr} \mathrm{r}^{3}}$ | $\frac{\mathrm{T}}{\mathrm{gr}{ }^{2}}$ | $\frac{5}{g r^{2}}$ | $\frac{\mathrm{M}}{\mathrm{gr} \mathrm{r}^{3}}$ | $\frac{1}{9 r^{2}}$ | $\frac{5}{9 r^{2}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 1 | +0.030 | -0.024 | 0 | +0.029 | -0.017 | 0 | +0.023 | -0.009 | 0 |
|  |  | 2 | +0.029 | -0.022 | +0.013 | +0.029 | -0.015 | +0.009 | +0.023 | -0.008 | +0.006 |
|  |  | 3 | +0.023 | 10.003 | +0.054 | +0.024 | +0.004 | +0.042 | +0.019 | +0.007 | +0.030 |
|  |  | 4 | +0.010 | +0.085 | +0.120 | +0.011 | +0.069 | +0.093 | +0.008 | +0.056 | +0.068 |
|  |  | 5 | -0.011 | +0.233 | +0.163 | -0.009 | +0.185 | +0.127 | -0.010 | +0.144 | +0.093 |
|  |  | 6 | -0.030 | +0.399 | +0.13 | -0.03 | +0.316 | +0.1 | -0.02 | +0. | +0. |
|  |  | 7 | -0.041 | +0.483 | +0.024 | -0.041 | +0.382 | +0.017 | -0.038 | +0.292 | $+0.009$ |
|  |  | 8 | -0.040 | +0.482 | -0.031 | -0.03 | +0.381 | -0.028 | -0.036 | +0.291 | -0.027 |
|  |  | 9 | 0.024 | +0.476 | -0.085 | -0.025 | +0.375 | -0.072 | -0.024 | +0.286 | -0.062 |
|  |  | 10 | +0.009 | +0.463 | -0.139 | +0.001 | +0.364 | -0.116 | -0.002 | +0.276 | -0.096 |
|  |  | 11 | $+0.013$ | +0.093 | -0.190 | +0.011 | +0.081 | -0.171 | +0.008 | +0.068 | -0.153 |
|  |  | 12 | +0.038 | +0.044 | -0.001 | +0.035 | +0.035 | -0.074 | +0.031 | +0.026 | -0.066 |
|  |  | 13 | +0.048 | +0.027 | -0.019 | +0.044 | +0.019 | -0.017 | +0.040 | +0.011 | -0.016 |
|  |  | 14 | $+0.049$ | +0.024 | 0 | +0.045 | +0.017 | 0 | +0.042 | +0.009 | 0 |


| (e) | POINT | $\frac{\mathrm{M}}{\mathrm{gr}}$ | $\frac{1}{g r^{2}}$ | $\frac{\mathrm{S}}{\mathrm{gr}}{ }^{2}$ | $\frac{\mathrm{M}}{\mathrm{gr}}$ | $\frac{1}{9 r^{2}}$ | $\frac{\mathrm{S}}{9 \mathrm{r}^{2}}$ | $\frac{\mathrm{M}}{\mathrm{gr}}{ }^{\text {3 }}$ | $\frac{1}{g r^{2}}$ | $\frac{5}{g r^{2}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1 | +0.029 | -0.031 | 0 | +0.030 | -0.023 | 0 | +0.026 | -0.016 | 0 |
|  | 2 | +0.028 | -0.028 | +0.014 | +0.029 | -0.021 | +0.011 | +0.025 | -0.014 | +0.008 |
|  | 3 | +0.021 | -0.002 | +0.058 | +0.023 | -0.000 | +0.045 | +0.020 | +0.001 | $+0.034$ |
|  | 4 | +0.006 | +0.001 | +0.124 | +0.010 | +0.065 | +0.097 | +0.008 | +0.051 | +0.073 |
|  | 5 | -0.016 | +0.230 | $+0.168$ | -0.012 | +0.182 | +0.132 | -0.011 | +0.140 | +0.100 |
|  | 6 | -0.037 | +0.397 | +0.138 | -0.034 | +0.314 | +0.108 | -0.032 | +0.241 | +0.081 |
|  | 7 | -0.050 | +0.483 | +0.031 | -0.047 | +0.382 | +0.023 | -0.043 | +0.292 | +0.016 |
|  | 8 | -0.051 | +0.483 | -0.028 | -0.046 | +0.381 | -0.025 | -0.043 | +0.292 | -0.022 |
|  | 9 | -0.033 | +0.476 | -0.087 | -0.031 | +0.375 | -0.073 | -0.031 | +0.286 | -0.060 |
|  | 10 | -0.029 | +0.254 | -0.249 | -0.025 | +0.213 | -0.210 | -0.025 | +0.174 | -0.172 |
|  | 11 | +0.005 | +0.138 | -0.152 | +0.007 | +0.112 | -0.126 | +0.005 | +0.088 | -0.102 |
|  | 12 | +0.025 | +0.071 | -0.073 | +0.026 | +0.055 | -0.058 | +0.022 | +0.040 | -0.045 |
|  | 13 | +0.031 | +0.036 | -0.018 | +0.032 | +0.027 | -0.014 | +0.028 | +0.019 | -0.011 |
|  | 14 | +0.032 | +0.031 | 0 | +0.033 | +0.023 | 0 | +0.028 | +0.016 | 0 |

NOTE: $g$ represents the weight per unit
volume of soil cover on the orch of the conduit section in units consistent with those of the rodius $r$


## SINGLE BARREL CONDUIT BEGGS DEFORMETER STRESS ANALYSIS COEFFICIENTS FOR MOMENT, THRUST, AND SHEAR VERTICAL ARCH LOAD-TRIANGULAR FOUNDATION REACTION

SHAPES A, B, AND C




| POINT | $\frac{M}{9 r^{2}}$ | $\frac{T}{a r}$ | $\frac{S}{9 r}$ | $\frac{\mathrm{M}}{\mathrm{ar}^{2}}$ | $\frac{T}{9 r}$ | $\frac{\mathrm{S}}{\mathrm{ar}}$ | $\frac{M}{9 r^{2}}$ | $\frac{T}{a r}$ | $\frac{s}{a r}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | -0.170 | $+0.447$ | 0 | -0.151 | +0.404 | 0 | -0.121 | +0.360 | 0 |
| 2 | -0.151 | +0.432 | -0.116 | -0.135 | +0.390 | -0.105 | -0.108 | $+0.347$ | -0.093 |
| 3 | -0.097 | +0.382 | -0.220 | -0.089 | $+0.345$ | -0.199 | -0.069 | $+0.307$ | -0.177 |
| 4 | -0.016 | +0.294 | -0.294 | -0.020 | +0.266 | -0.266 | -0.011 | +0.237 | -0.237 |
| 5 | +0.075 | +0.177 | -0.306 | +0.059 | +0.161 | -0.278 | +0.057 | +0.144 | -0.249 |
| 6 | +0.155 | +0.062 | 0.233 | +0.130 | $+0.057$ | -0.214 | +0.119 | $+0.052$ | -0.193 |
| 7 | +0.202 | 0 | -0.072 | $+0.172$ | 0 | -0.071 | +0.156 | - 0 | -0.068 |
| 8 | +0.200 | $+0.007$ | +0.102 | +0.170 | +0.007 | +0.099 | +0.153 | +0.007 | +0.097 |
| 9 | +0.145 | + 0.041 | +0.309 | +0.116 | +0.041 | $+0.303$ | $\pm 0.099$ | +0.041 | +0.300 |
| 10 | +0.031 | + 0.110 | $+0.549$ | +0.002 | $+0.111$ | +0.543 | -0.015 | $+0.112$ | +0.539 |
| 11 | -0.083 | + 1.032 | +0.207 | -0.088 | +0.910 | +0.186 | -0.080 | $+0.790$ | +0.165 |
| 12 | -0.135 | $+1.043$ | +0.139 | -0.131 | +0.921 | $\pm 0.125$ | -0.119 | +0.799 | +0.110 |
| 13 | -0.166 | +1.050 | $+0.069$ | -0.162 | +0.927 | $+0.062$ | -0.143 | +0.805 | $+0.055$ |
| 14 | -0.176 | + 1.053 | 0 | -0.171 | +0.929] | 0 | -0.151 | +0.807 | 0 |


| POint | $\frac{M}{a r^{2}}$ | $\frac{T}{q^{\prime}}$ | $\frac{s}{4 r}$ | $\frac{\mathrm{M}}{9 r^{2}}$ | $\frac{1}{q r}$ | $\frac{5}{9 r}$ | $\frac{M}{a r^{2}}$ | $\frac{\mathrm{T}}{\mathrm{q}}$ | $\frac{s}{4 r}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | -0.162 | +0.431 | 0 | -0.139 | +0.391 | , | 0.114 | +0.347 | 0 |
| 2 | -0.144 | +0.416 | -0.112 | -0.123 | +0.377 | -0.101 | -0.101 | +0.335 | 0.090 |
| 3 | -0.092 | +0.368 | -0.212 | -0.079 | +0.334 | -0.193 | -0.064 | +0. 296 | -0.171 |
| 4 | -0.015 | +0.282 | -0.282 | -0.012 | +0.256 | -0.256 | -0.008 | +0.228 | -0.228 |
| 5 | +0.072 | $+0.169$ | -0.292 | +0.064 | +0.154 | -0.267 | +0.057 | +0.137 | -0.237 |
| 6 | +0.148 | $+0.058$ | 0.218 | +0.131 | +0.054 | -0.201 | +0.115 | +0.048 | 80 |
| 7 | +0.189 | , | -0.056 | +0.169 | 0 | -0.058 | +0.148 | 0 | -0.055 |
| 8 | +0.184 | +0.014 | +0.119 | +0.165 | +0.013 | +0.109 | +0.144 | +0.012 | +0.102 |
| 9 | +0.131 | +0.076 | +0.327 | +0.115 | +0.074 | $+0.308$ | +0.096 | +0.073 | +0.293 |
| 10 | +0.03 | +0.202 | +0.567 | +0.019 | +0.199 | +0.540 | $\pm 0.002$ | +0.197 | $+0.517$ |
| 11 | -0.053 | +1.007 | +0.358 | -0.051 | +0.884 | +0.325 | -0.053 | +0.766 | +0.292 |
| 12 | -0.139 | +1.041 | +0.242 | -0.127 | +0.916 | +0. 219 | -0.119 | +0.796 | +0. 197 |
| 13 | -0.192 | $+1.062$ | +0.122 | -0.174 | +0.936 | +0.110 | -0.159 | +0.814 | 0. |
| 14 | -0.209 | $+1.069$ | 0 | -0.189 | +0.942 | 0 | -0.173 | +0.820 | 0 |


| POINT | $\frac{\mathrm{M}}{\mathrm{ar}^{2}}$ | $\frac{T}{a r}$ | $\frac{5}{9 r}$ | $\frac{M}{9 r^{2}}$ | $\frac{\mathrm{I}}{9 r}$ | $\frac{s}{\text { ar }}$ | $\frac{M}{a r^{2}}$ | $\frac{1}{\text { ar }}$ | $\frac{5}{9 r}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | -0.172 | +0.442 | 0 | -0.143 | +0.400 | 0 | -0.122 | +0.364 | 0 |
| 2 | -0.153 | +0.426 | 0.114 | -0.127 | +0.386 | -0.104 | 0.10 | +0.352 | -0.094 |
| 3 | -0.100 | +0.377 | -0.218 | -0.081 | +0.341 | -0.197 | -0.07 | +0.31 | -0.180 |
| 4 | -0.020 | +0.290 | -0.290 | -0.013 | +0.263 | -0.263 | -0.011 | +0.240 | -0.240 |
| 5 | +0.069 | +0.174 | -0.301 | +0.065 | +0.158 | -0.274 | +0.0 | +0.1 | -0.252 |
| 6 | +0. | +0.061 | -0.228 | +0.135 | +0.056 | -0.210 | +0.121 | +0.053 | -0.197 |
| 7 | +0.193 | 0 | -0.067 | +0.176 | 0 | -0.067 | +0.159 | 0 | -0.073 |
| 8 | +0. 189 | +0.015 | +0.121 | +0.173 | +0.014 | +0.11 | +0.158 | $+0.013$ | +0.096 |
| 9 | +0.132 | $+0.086$ | +0.345 | +0.118 | +0.084 | +0.327 | +0.107 | + 0.081 | +0.302 |
| 10 | +0.093 | +0.477 | +0.409 | +0.088 | +0.405 | +0.352 | +0.088 | +0.329 | +0.292 |
| 11 | -0.003 | +0.637 | +0.423 | +0.002 | +0.552 | +0.370 | +0.013 | +0.463 | $+0.313$ |
| 12 | -0.099 | +0.809 | +0.404 | -0.087 | +0.712 | +0.356 | -0.068 | +0.610 | +0.305 |
| 13 | -0.167 | +0.991 | +0.234 | -0.149 | +0.874 | +0.206 | -0.123 | +0.751 | +0.177 |
| 14 | -0.192 | +1.058 | - | -0.172 | $+0.933$ | $\bigcirc$ | -0.143 | +0.802 | , |

# SINGLE BARREL CONDUIT BEGGS DEFORMETER STRESS ANALYSIS COEFFICIENTS FOR MOMENT, THRUST, AND SHEAR TRIANGULAR HORIZONTAL LOAD - BOTH SIDES <br> SHAPES A, B, AND C 



| POINT | $\frac{M}{p r^{2}}$ | $\frac{\mathrm{T}}{\mathrm{pr}}$ | $\frac{\mathrm{S}}{\mathrm{pr}}$ | $\frac{\mathrm{m}}{\mathrm{pr}}{ }^{2}$ | $\frac{T}{\text { Dr }}$ | $\frac{\mathrm{S}}{\mathrm{pr}}$ | $\frac{\mathrm{M}}{\mathrm{pr}}{ }^{2}$ | $\frac{T}{\text { pr }}$ | $\frac{S}{\text { pr }}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +0.000 | -1.006 | 0 | +0.000 | -1.005 | 0 | +0.000 | -1.004 | 0 |
| 2 | 0 | -1.006 | +0.001 | -0.000 | -1.005 | $+0.001$ | -0.000 | -1.003 | +0.001 |
| 3 | -0.001 | -1.005 | +0.003 | -0.001 | -1.004 | $+0.003$ | -0.000 | -1.003 | $+0.002$ |
| 4 | -0.002 | -1.004 | +0.004 | -0.002 | -1.004 | $+0.004$ | -0.001 | -1.003 | +0.003 |
| 5 | -0.003 | $-1.003$ | +0.005 | -0.003 | -1.003 | $+0.004$ | -0.002 | -1.002 | $+0.003$ |
| 6 | -0.005 | -1.001 | +0.006 | -0.004 | -1.001 | +0.005 | -0.003 | -1.001 | $+0.003$ |
| 7 | -0.007 | $-1.000$ | +0.006 | -0.006 | $-1.000$ | +0.005 | -0.004 | -1.000 | $+0.004$ |
| 8 | +0.009 | -0.988 | -0.147 | +0.011 | -0.988 | -0.144 | +0.014 | -0.989 | -0.142 |
| 9 | +0.058 | -0.951 | -0.298 | +0.063 | -0.954 | -0.291 | +0.068 | -0.957 | -0.285 |
| 10 | +0.057 | -0.948 | +0.308 | +0.062 | -0.951 | +0.300 | +0.067 | -0.955 | +0.290 |
| 11 | +0.005 | -0.983 | $+0.156$ | +0.008 | -0.985 | +0.152 | +0.012 | -0.986 | +0.147 |
| 12 | -0.013 | -0.995 | +0.003 | -0.011 | -0.995 | +0.002 | -0.007 | -0.997 | +0.002 |
| 13 | -0.014 | -0.994 | +0.001 | -0.012 | -0.995 | +0.001 | -0.008 | -0.997 | $+0.001$ |
| 14 | -0.014 | -0.994 | 0 | -0.012 | -0.995 | 0 | -0.008 | -0.996 | 0 |


| $p O 1 N T$ | $\frac{M}{p r^{2}}$ | $\frac{T}{p r}$ | $\frac{S}{p r}$ | $\frac{M}{p r^{2}}$ | $\frac{r}{p r}$ | $\frac{S}{p r}$ | $\frac{M}{p r}$ | $\frac{T}{p r}$ | $\frac{S}{p r}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +0.007 | -1.019 | 0 | +0.005 | -1.017 | 0 | +0.003 | -1.012 | 0 |
| 2 | +0.006 | -1.019 | +0.005 | +0.005 | -1.016 | +0.004 | +0.003 | -1.011 | +0.003 |
| 3 | +0.003 | -1.017 | +0.010 | +0.003 | -1.015 | +0.008 | +0.002 | -1.010 | +0.006 |
| 4 | -0.000 | -1.014 | +0.014 | -0.001 | -1.012 | +0.012 | -0.000 | -1.008 | +0.008 |
| 5 | -0.005 | -1.010 | +0.017 | -0.005 | -1.008 | +0.015 | -0.003 | -1.006 | +0.010 |
| 6 | -0.011 | -1.005 | +0.019 | -0.009 | -1.004 | +0.016 | -0.006 | -1.003 | +0.011 |
| 7 | -0.018 | -1.000 | +0.019 | -0.014 | -1.000 | +0.017 | -0.009 | -1.000 | +0.012 |
| 8 | -6.007 | -0.987 | -0.126 | -0.002 | -0.988 | -0.126 | +0.005 | -0.989 | -0.127 |
| 9 | +0.033 | -0.953 | -0.270 | +0.041 | -0.955 | -0.267 | +0.051 | -0.958 | -0.264 |
| 10 | +0.096 | -0.897 | -0.411 | +0.108 | -0.902 | -0.404 | +0.123 | -0.909 | -0.398 |
| 11 | +0.091 | -0.885 | +0.436 | +0.105 | -0.892 | +0.425 | +0.121 | -0.902 | +0.413 |
| 12 | +0.021 | -0.938 | +0.294 | +0.031 | -0.943 | +0.287 | +0.045 | -0.950 | +0.279 |
| 13 | -0.025 | -0.970 | +0.148 | -0.017 | -0.973 | +0.144 | -0.005 | -0.979 | +0.140 |
| 14 | -0.042 | -0.981 | 0 | -0.034 | -0.983 | 0 | -0.022 | -0.988 | 0 |


| POINT | $\frac{M}{p r^{2}}$ | $\frac{T}{p r}$ | $\frac{s}{p r}$ | $\frac{M}{p r^{2}}$ | $\frac{T}{p r}$ | $\frac{S}{p r}$ | $\frac{M}{p r^{2}}$ | $\frac{T}{p r}$ | $\frac{S}{p r}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +0.022 | -1.053 | 0 | +0.017 | -1.046 | 0 | +0.011 | -1.033 | 0 |
| 2 | +0.020 | -1.052 | +0.014 | +0.015 | -1.044 | +0.012 | +0.009 | -1.032 | +0.008 |
| 3 | +0.013 | -1.046 | +0.027 | +0.010 | -1.040 | +0.023 | +0.006 | -1.028 | +0.016 |
| 4 | +0.003 | -1.038 | +0.038 | +0.001 | -1.032 | +0.032 | +0.000 | -1.023 | +0.023 |
| 5 | -0.011 | -1.027 | +0.046 | -0.010 | -1.023 | +0.040 | -0.007 | -1.016 | +0.028 |
| 6 | -0.027 | -1.014 | +0.052 | -0.023 | -1.012 | +0.044 | -0.016 | -1.008 | +0.032 |
| 7 | -0.046 | -1.000 | +0.053 | -0.037 | -1.000 | +0.046 | -0.025 | -1.000 | +0.033 |
| 8 | -0.031 | -0.993 | -0.163 | -0.021 | -0.994 | -0.169 | -0.004 | -0.995 | -0.181 |
| 9 | +0.041 | -0.979 | -0.379 | +0.055 | -0.981 | -0.384 | +0.075 | -0.984 | -0.394 |
| 10 | +0.170 | -0.958 | -0.592 | +0.187 | -0.962 | -0.596 | +0.212 | -0.967 | -0.606 |
| 11 | +0.155 | -0.916 | +0.655 | +0.178 | -0.926 | +0.650 | +0.207 | -0.942 | +0.644 |
| 12 | +0.008 | -0.933 | +0.438 | +0.030 | -0.942 | +0.435 | +0.060 | -0.956 | +0.431 |
| 13 | -0.081 | -0.943 | +0.220 | -0.060 | -0.951 | +0.218 | -0.030 | -0.964 | +0.216 |
| 14 | -0.111 | -0.947 | 0 | -0.090 | -0.954 | 0 | -0.060 | -0.967 | 0 |

SINGLE BARREL CONDUIT BEGGS DEFORMETER STRESS ANALYSIS
COEFFICIENTS FOR MOMENT, THRUST, AND SHEAR
UNIFORM INTERNAL RADIAL LOAD
SHAPES A, B, AND $c$


| Pressure distribution along vertical \& of conduit- | $t=\frac{r}{2}$ |  |  |  | $t=\frac{5}{3}$ |  |  | $t=\frac{1}{6}$ |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | POINT | $\frac{m}{4 r^{3}}$ | $\frac{\mathrm{T}}{}{ }^{\text {r }}$ | $\frac{\mathrm{s}}{\mathrm{wr}}{ }^{2}$ | $\frac{\mathrm{m}}{\mathrm{m} \mathrm{r}^{3}}$ | $\frac{\mathrm{T}}{\mathrm{m} \mathrm{r}^{\text {e }}}$ | $\frac{3}{m^{2}}$ | $\frac{\mathrm{m}}{\mathrm{m}{ }^{\text {r }}}$ | $\frac{T}{w r^{2}}$ | $\frac{3}{\mathrm{wr}^{2}}$ |
|  | 1 | +0.281 | -0.687 | 0 | +0.266 | -0.691 | 0 | ${ }^{+0.239}$ | -0.693 | 0 |
|  | 2 | +0.252 | -0.664 | +0.175 | +0.239 | -0.667 | $+0.176$ | +0.214 | -0.669 | +0. |
|  | 3 | +0.169 | -0.598 | +0.320 | +0.162 | -0.601 | +0.322 | +0.142 | -0.603 | $+0.323$ |
| 4 x | 4 | +0.048 | -0.501 | +0.410 | +0.048 | -0.504 | +0.413 | +0.036 | -0.505 | +0.414 |
|  | 5 | -0.090 | -0.390 | +0.424 | -0.083 | -0.302 | +0.427 | -0.085 | -0.393 | +0.429 |
|  | 6 | -0.220 | -0.287 | +0.350 | -0.204 | -0.288 | +0.354 | -0.199 | -0.288 | +0.355 |
| - | 7 | -0.310 | -0.215 | +0.187 | -0.290 | -0.815 | +0.191 | -0.278 | -0.215 | +0. 193 |
|  | 8 | -0.325 | -0.199 | -0.121 | -0.304 | -0.199 | -0.117 | -0.293 | -0.199 | -0.115 |
| + | 9 | -0.245 | -0.174 | -0.508 | -0.224 | -0.175 | -0.503 | 0.210 | -0.176 | -0.501 |
| $\underline{-10}$ | 10 | -0.055 | -0.132 | -0.967 | -0.031 | -0.135 | -0.963 | -0.014 | -0.138 | -0.961 |
|  | 11 | +0.171 | -0.986 | -0.040 | +0.149 | -0.970 | -0. 103 | ${ }^{+0.114}$ | -0.954 | -0.169 |
|  | 12 | +0.218 | -1.147 | -0.158 | +0.212 | -1.133 | -0.229 | +0.195 | -1.120 | -0.311 |
| ) | 13 | +0.271 | -1.266 | -0.148 | ${ }^{+0.281}$ | -1.259 | -0.196 | +0.292 | -1.253 | 0.25 |
|  | 14 | +0.297 | -1.313 | $\bigcirc$ | +0.314 | 1.309 | 0 | +0.324 | -1.307 | 0 |


|  | Pressure distribution along vertical \& of conduit- | POINT | $\frac{m}{w r^{3}}$ | $\frac{\mathrm{T}}{\mathrm{w} \mathrm{r}^{2}}$ |  | $\frac{\mathrm{M}}{\mathrm{wr}}$ | $\frac{T}{\text { w } r^{2}}$ | $\frac{3}{w r^{2}}$ | $\frac{\mathrm{m}}{\mathrm{wr}}{ }^{3}$ | $\frac{T}{T r^{2}}$ | $\frac{3}{m r^{2}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 1 | +0.259 | -0.645 | 0 | +0.237 | -0.654 | 0 | +0.219 | -0.660 | 0 |
|  |  | 2 | +0.232 | -0.623 | +0.164 | +0.211 | -0.632 | +0.166 | +0.195 | -0.637 | +0.168 |
|  |  | 3 | +0.155 | -0.561 | +0.299 | +0.138 | -0.570 | +0.304 | +0.127 | -0.574 | +0.307 |
|  |  | 4 | +0.042 | -0.471 | +0.380 | +0.031 | -0.478 | +0.387 | +0.026 | -0.482 | +0.391 |
|  |  | 5 | -0.086 | -0.369 | +0.387 | -0.091 | -0.374 | +0.396 | -0.088 | -0.376 | +0.400 |
|  |  | 6 | -0.202 | -0.276 | +0.309 | -0.202 | -0.278 | +0.319 | -0.192 | -0.280 | +0.324 |
|  |  | 7 | -0.279 | -0.215 | +0.145 | -0.277 | -0.215 | +0.154 | -0.263 | -0.215 | +0.160 |
|  | $\times 1$ | 8 | -0.289 | -0.191 | -0.125 | -0.288 | -0.190 | -0.115 | -0.273 | -0.190 | -0.109 |
|  |  | 9 | -0.230 | -0.144 | -0.457 | -0.227 | -0.145 | -0.447 | -0.210 | -0.147 | -0.441 |
|  | 14.15 | 10 | -0.102 | -0.064 | -0.840 | -0.092 | -0.070 | -0.830 | -0.069 | -0.078 | -0.824 |
|  |  | 11 | +0.106 | -0.752 | -0.309 | +0.076 | -0.725 | -0.355 | +0.059 | -0.699 | -0.404 |
|  |  | 12 | +0.231 | -1.051 | -0.332 | +0.204 | -1.027 | -0.383 | +0.192 | -1.005 | -0.443 |
|  | $1)^{\circ}$ | 13 | +0.324 | -1.271 | -0.230 | +0.304 | -1.256 | -0.266 | +0.300 | -1.245 | -0.309 |
|  |  | 14 | +0.362 | -1.355 | 0 | +0.345 | -1.346 | 0 | +0.346 | -1.340 | 0 |


|  | Pressure distribution olong vertical $\&$ of conduit-. | POINT | $\frac{m}{m r^{3}}$ | $\frac{T}{w^{2}}$ | $\frac{5}{w r^{2}}$ | $\frac{M}{m r^{3}}$ | $\frac{T}{w r^{2}}$ | $\frac{\text { S }}{\text { w }}$ | $\frac{M}{m}$ | $\frac{T}{\text { m } r^{8}}$ | $\frac{3}{1 r^{2}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 1 | +0.265 | -0.654 | 0 | +0.241 | -0.660 | 0 | +0.234 | -0.677 | 0 |
|  |  | 2 | +0.237 | -0.632 | +0.166 | +0.215 | -0.638 | +0.168 | +0.209 | -0.655 | +0.172 |
|  | 4 | 3 | +0.159 | -0.570 | $+0.304$ | +0.141 | -0.575 | +0.307 | +0.139 | -0.590 | +0.315 |
|  |  | 4 | +0.045 | -0.478 | +0.387 | +0.033 | -0.482 | +0.391 | +0.035 | -0.494 | $+0.403$ |
|  |  | 5 | -0.086 | -0.374 | +0.395 | -0.090 | -0.377 | +0.401 | -0.083 | -0.385 | $+0.416$ |
|  |  | 6 | -0.205 | -0.278 | +0.318 | -0.203 | -0.280 | +0.324 | -0.192 | -0.284 | $+0.341$ |
|  | $\pm$ - 4 | 7 | -0.284 | -0.215 | +0.154 | -0.279 | -0.215 | +0.160 | -0.268 | -0.215 | +0.177 |
|  | - -6 | 8 | -0.296 | -0.188 | -0.134 | -0.291 | -0.187 | -0.127 | -0.282 | -0.186 | -0.109 |
|  |  | 9 | -0.229 | -0.135 | -0.492 | -0.221 | -0.138 | -0.484 | -0.212 | -0.137 | -0.467 |
|  | - 2wn | 10 | -0.151 | -0.416 | -0.298 | -0.163 | -0.402 | -0.305 | -0.175 | -0.380 | -0.305 |
|  |  | 11 | -0.017 | -0.667 | -0.545 | -0.031 | -0.652 | -0.555 | -0.047 | -0.627 | -0.561 |
|  |  | 12 | +0.149 | -0.925 | -0.699 | +0.139 | -0.920 | -0.697 | +0.126 | -0.904 | -0.689 |
|  |  | 13 | +0.288 | -1.216 | -0.474 | +0.278 | -1.210 | -0.472 | +0.264 | -1.194 | -0.469 |
|  |  | 14 | +0.344 | -1.346 | 0 | +0.335 | -1.340 | 0 | +0.321 | -1.323 | 0 |

NOTE: $w$ represents the weight per unit volume of water in units consistent
with those of the rodius $r$.


## SINGLE BARREL CONDUIT

 BEGGS DEFORMETER STRESS ANALYSIS COEFFICIENTS FOR MOMENT, THRUST, AND SHEAR TRIANGULAR INTERNAL RADIAL LOAD - TRIANGULAR FOUNDATION REACTION SHAPES A, B, AND C
$t=\frac{r}{2}$
$t=\frac{r}{3}$
$t=\frac{r}{6}$

| INT | $\frac{\mathrm{M}}{\mathrm{vr}}$ | Tr | $\frac{S}{v r}$ | $\frac{\mathrm{M}}{\mathrm{Vr}}$ | $\frac{T}{v r}$ | $\frac{S}{\text { a }}$ | $\frac{M}{v r^{2}}$ | $\frac{T}{v r}$ | $\frac{S}{\text { v }}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +0.393 | 0 | 0 | +0.348 | 0 | 0 | +0.302 | 0 | 0 |
| 2 | +0.352 | +0.036 | +0.316 | +0.310 | +0.035 | +0.299 | +0.265 | +0.034 | +0.280 |
| 3 | +0.232 | +0.146 | +0.628 | +0.196 | +0.142 | +0.593 | +0.158 | $+0.138$ | +0.556 |
| 4 | $+0.043$ | +0 332 | +0.932 | +0.017 | +0.324 | +0.880 | -0.010 | +0.315 | +0.824 |
| 5 | -0137 | +1.413 | +0 503 | -0.123 | +1.251 | +0.460 | -0.111 | $+1.090$ | +0.416 |
| 6 | -0.259 | $+1.461$ | +0.339 | -0.23 | +1.297 | +0.310 | -0.205 | +1.132 | +0.281 |
| 7 | -0.332 | $+1.490$ | +0 171 | -0.296 | +1.324 | +0.156 | -0.262 | +1.158 | +0.1 |
| 8 | -0.357 | $+1.500$ | 0 | -0.318 | +1.333 | 0 | -0.282 | +1.167 | 0 |
| 9 | -0.332 | +i490 | -0.171 | -0.296 | +1.324 | -0.156 | -0.262 | +1.158 | 0.141 |
| 10 | -0.259 | $+1461$ | -0.339 | -0.2 | +1.297 | -0.310 | -0.205 | +1.132 | -0.281 |
| 1 | -0.137 | $+1.413$ | -0.503 | -0.123 | +1.251 | -0.460 | -0.111 | +1.090 | -0.416 |
| 12 | +0.043 | +0.332 | -0.932 | +0.017 | +0.324 | -0.880 | -0.010 | +0.315 | -0.824 |
| 13 | +0.232 | +0.146 | -0.628 | +0.196 | +0.142 | -0.593 | +0.158 | +0.138 | -0.556 |
| 14 | +0.352 | $+0.036$ | -0316 | +0.310 | +0.035 | -0.299 | +0.265 | +0.034 | -0.28 |
| 5 | +0.393 | 0 | 0 | +0.348 | 0 | 0 | +0.302 | 0 | 0 |



| POINT | $\frac{\mathrm{M}}{\mathrm{vr}}{ }^{2}$ | $\frac{T}{v r}$ | $\frac{S}{v r}$ | $\frac{\mathrm{m}}{\mathrm{v}} \mathrm{r}^{2}$ | $\frac{\mathrm{T}}{\mathrm{vr}}$ | $\underline{\mathrm{S}}$ | $\frac{\mathrm{m}}{\mathrm{m}}$ | $\frac{T}{v r}$ | $\frac{\mathrm{S}}{\mathrm{vr}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +0.353 | +0.066 | 0 | +0.317 | +0.060 | 0 | +0.279 | +0.054 | 0 |
| 2 | +0.306 | +0.165 | +0.358 | +0.275 | +0.147 | +0.318 | +0.242 | +0.131 | +0.278 |
| 3 | +0.177 | +0.433 | +0.610 | $+0.160$ | +0.385 | +0.547 | $+0.141$ | +0.339 | +0.478 |
| 4 | +0.003 | +0.797 | +0.703 | +0.004 | +0.709 | +0.624 | +0.005 | +0.622 | $+0.545$ |
| 5 | -0.168 | +1.158 | +0.592 | -0.148 | +1.030 | +0.525 | -0.129 | +0.902 | +0.458 |
| 6 | -0.285 | $+1.417$ | +0.311 | -0.253 | +1.260 | +0 275 | -0.221 | $+1.103$ | +0.239 |
| 7 | -0.314 | $+1.500$ | -0.066 | -0.280 | + +1.333 | -0.060 | -0.245 | $+1.167$ | -0.054 |
| 8 | -0.292 | $+1.500$ | -0.066 | -0.260 | +1.333 | -0.060 | -0.227 | $+1.167$ | -0.054 |
| 9 | -0.269 | $+1.500$ | -0.066 | -0.240 | +1.333 | -0.060 | -0.209 | +1.167 | -0.054 |
| 10 | -0.247 | $+1.500$ | -0.066 | -0.220 | +1.333 | -0.060 | -0.191 | $+1.167$ | -0.054 |
| 11 | +0.019 | -0.066 | -1.000 | -0.043 | -0.060 | -1.000 | -0.103 | -0.054 | -1.000 |
| 12 | +0.297 | -0.066 | -0.667 | +0.235 | -0.060 | -0.667 | +0.175 | -0.054 | -0.667 |
| 13 | +0.464 | -0.066 | -0.333 | +0.401 | -0.060 | -0.333 | +0.341 | -0.054 | -0.333 |
| 14 | +0.519 | -0.066 | 0 | +0.457 | -0.060 | 0 | $+0.397$ | -0.054 | 0 |


| POINT | $\frac{M}{v r^{2}}$ | $\frac{T}{v r}$ | $\frac{S}{v r}$ | $\frac{M}{v r^{2}}$ | $\frac{T}{v r}$ | $\frac{S}{v r}$ | $\frac{M}{v r^{2}}$ | $\frac{T}{v r}$ | $\frac{S}{v r}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +0.371 | +0.019 | 0 | +0.331 | +0.018 | 0 | +0.290 | +0.017 | 0 |
| 2 | +0.322 | +0.119 | +0.370 | +0.287 | +0.107 | +0.329 | +0.251 | +0.095 | +0.287 |
| 3 | +0.187 | +0.391 | +0.640 | +0.167 | +0.349 | +0.568 | +0.146 | +0.306 | +0.497 |
| 4 | +0.003 | +0.763 | +0.737 | +0.003 | +0.679 | +0.654 | +0.003 | +0.595 | +0.571 |
| 5 | -0.180 | +1.135 | +0.633 | -0.159 | +1.009 | +0.562 | -0.139 | +0.884 | +0.490 |
| 6 | -0.311 | +1.404 | +0.357 | -0.276 | +1.249 | +0.316 | -0.241 | +1.093 | +0.275 |
| 7 | -0.355 | +1.500 | -0.019 | -0.315 | +1.333 | -0.018 | -0.275 | +1.167 | -0.017 |
| 8 | -0.322 | +1.458 | -0.227 | -0.285 | +1.294 | -0.202 | -0.248 | +1.131 | -0.178 |
| 9 | -0.238 | +1.340 | -0.409 | -0.209 | +1.186 | -0.363 | -0.180 | +1.033 | -0.318 |
| 10 | -0.114 | +1.158 | -0.543 | -0.096 | +1.020 | -0.480 | -0.078 | +0.881 | -0.417 |
| 11 | -0.008 | +0.406 | -0.945 | -0.026 | +0.379 | -0.882 | -0.043 | +0.351 | -0.820 |
| 12 | +0.222 | +0.176 | -0.675 | +0.189 | +0.164 | -0.630 | +0.156 | +0.152 | -0.585 |
| 13 | +0.369 | +0.031 | -0.351 | +0.326 | +0.029 | -0.328 | +0.283 | +0.026 | -0.305 |
| 14 | +0.419 | -0.019 | 0 | +0.372 | -0.017 | 0 | +0.326 | -0.017 | 0 |


SINGLE BARREL CONDUIT BEGGS DEFORMETER STRESS ANALYSIS COEFFICIENTS FOR MOMENT, THRUST, AND SHEAR UNIFORM VERTICAL LOAD - UNIFORM FOUNDATION REACTION SHAPES $D, E, A N D F$


## SINGLE BARREL CONDUIT

 BEGGS DEFORMETER STRESS ANALYSIS COEFFICIENTS FOR MOMENT, THRUST, AND SHEAR concentrated vertical load - uniform foundation reactionShapes $D, E$, AND $F$



| POINT | $\frac{M}{v r^{2}}$ | $\frac{T}{v r}$ | $\frac{S}{v r}$ | $\frac{M}{v r^{2}}$ | $\frac{T}{v r}$ | $\frac{s}{v r}$ | $\frac{M}{v r^{2}}$ | $\frac{\mathrm{T}}{\mathrm{vr}}$ | $\frac{S}{v r}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +0.491 | +0.216 | 0 | +0.416 | +0.193 | 0 | +0.340 | +0.173 | 0 |
| 2 | +0.417 | +0.279 | +0.541 | +0.347 | +0.254 | +0.508 | +0.276 | +0.232 | 0.472 |
| 3 | +0.223 | +0.439 | $+0.938$ | +0.165 | +0.407 | +0.870 | +0.109 | +0.376 | +0.798 |
| 4 | -0.054 | +0.648 | $+1.177$ | -0.089 | +0.601 | +1.074 | -0.121 | $+0.553$ | +0.964 |
| 5 | -0.328 | +1.485 | $+0.300$ | -0.285 | +1.318 | $+0.279$ | -0.249 | +1.152 | +0.254 |
| 6 | -0.390 | $+1.510$ | +0.129 | -0.342 | +1.342 | $+0.123$ | -0.299 | +1.174 | +0.113 |
| 7 | -0.402 | $+1.515$ | -0.044 | -0.354 | +1.347 | -0.035 | -0.310 | $+1.179$ | -0.030 |
| 8 | -0.365 | $+1.500$ | -0.216 | -0.322 | +1.333 | -0.193 | -0.283 | +1.167 | -0.173 |
| 9 | -0.278 | +1.466 | -0.385 | -0.246 | +1.302 | -0.348 | -0.217 | $+1.137$ | -0.313 |
| 10 | -0.144 | $+1.412$ | -0.549 | -0.128 | $+1.252$ | -0.498 | -0.114 | +1.091 | -0.448 |
| 11 | +0.038 | +1.341 | -0.706 | +0.032 | +1.185 | -0.641 | +0.025 | +1.028 | -0.577 |
| 12 | +0.125 | +0.016 | -0.687 | +0.115 | +0.047 | -0.685 | +0.098 | +0.076 | -0.685 |
| 13 | +0.226 | -0.147 | -0.319 | +0.219 | -0.122 | -0.316 | +0.206 | -0.100 | -0.315 |
| 14 | +0.271 | -0.206 | -0.092 | +0.265 | -0.184 | -0.090 | +0.252 | -0.163 | -0.089 |
| 15 | +0.279 | -0.216 | 0 | +0.273 | -0.193 | 0 | +0.26 | -0.173 | 0 |



| POINT | $\frac{M}{v r^{2}}$ | $\frac{T}{v r}$ | $\frac{S}{v r}$ | $\frac{\mathrm{M}}{\mathrm{V} \mathrm{r}^{2}}$ | $\frac{T}{v r}$ | $\frac{5}{v r}$ | $\frac{M}{v r^{2}}$ | $\frac{\mathrm{T}}{\mathrm{vr}}$ | $\frac{S}{v r}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +0.469 | +0.242 | 0 | +0.419 | +0.207 | 0 | +0.366 | +0.168 | 0 |
| 2 | +0.385 | +0.408 | +0.590 | +0.344 | +0.355 | +0.527 | +0.300 | +0.298 | +0.464 |
| 3 | +0.181 | +0.772 | +0.853 | +0.164 | +0.679 | +0.763 | +0.143 | $+0.583$ | +0.674 |
| 4 | -0.060 | +1.141 | +0.799 | -0.046 | +1.008 | $+0.716$ | -0.038 | +0.873 | +0.636 |
| 5 | -0.261 | +1.397 | +0.527 | -0.220 | +1.237 | $+0.476$ | -0.187 | +1.076 | +0.428 |
| 6 | -0.369 | +1.510 | +0.154 | -0.313 | $+1.340$ | +0.145 | -0.267 | +1.169 | +0.140 |
| 7 | -0.354 | $+1.500$ | -0.242 | -0.303 | +1.333 | -0.207 | -0.262 | $+1.167$ | -0.168 |
| 8 | -0.273 | $+1.500$ | -0.242 | -0.234 | +1.333 | -0.207 | -0.207 | +1.167 | -0.168 |
| 9 | -0.193 | $+1.500$ | -0.242 | -0.165 | +1.333 | -0.207 | -0.151 | $+1.167$ | -0.168 |
| 10 | -0.112 | $+1.500$ | -0.242 | -0.096 | +1.333 | -0.207 | -0.095 | +1.167 | -0.168 |
| 11 | +0.101 | -0.242 | -0.667 | +0.059 | -0.207 | -0.750 | -0.010 | -0.168 | -0.857 |
| 12 | +0.257 | -0.242 | -0.296 | +0.235 | -0.207 | -0.333 | +0.191 | -0.166 | -0.381 |
| 13 | +0.315 | -0.242 | -0.074 | +0.300 | -0.207 | -0.083 | +0.265 | -0.168 | -0.095 |
| 14 | 40.323 | -0.242 | 0 | +0.309 | -0.207 | 0 | +0.276 | -0.168 | 0 |



| POINT | $\frac{M}{v r^{2}}$ | $\frac{T}{v r}$ | $\frac{S}{v r}$ | $\frac{M}{v r^{2}}$ | $\frac{\mathrm{T}}{\mathrm{vr}}$ | $\frac{s}{v r}$ | $\frac{M}{V r^{2}}$ | $\frac{T}{v r}$ | $\frac{S}{v r}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +0.483 | +0.222 | 0 | +0.424 | +0.184 | 0 | +0.365 | +0.148 | 0 |
| 2 | +0.398 | +0.389 | +0.596 | +0.348 | +0.333 | +0.533 | +0.298 | +0.279 | +0.470 |
| 3 | +0.192 | +0.754 | +0.863 | +0.166 | 40.659 | +0.774 | +0.139 | +0.566 | +0.684 |
| 4 | -0.054 | +1.126 | $+0.813$ | -0.049 | +0.992 | +0.732 | -0.045 | +0.859 | +0.650 |
| 5 | -0.260 | +1.387 | +0.545 | -0.228 | +1.226 | +0.495 | -0.198 | +1.066 | +0.445 |
| 6 | -0.374 | $+1.505$ | +0.174 | -0.327 | +1.334 | +0.167 | -0.284 | $+1.164$ | +0.159 |
| 7 | -0.365 | $+1.500$ | -0.222 | -0.324 | +1.333 | -0.184 | -0.285 | +1.167 | -0.148 |
| 8 | -0.274 | +1.405 | -0.424 | -0.248 | $+1.248$ | -0.363 | -0.221 | +1.092 | -0.305 |
| 9 | -0.148 | +1.194 | -0.578 | -0.136 | +1.057 | -0.499 | -0.123 | +0.920 | -0.421 |
| 10 | -0.001 | +0.902 | -0.650 | -0.003 | +0.792 | -0.559 | -0.005 | +0.682 | -0.470 |
| 11 | +0.073 | +0.088 | -0.734 | +0.051 | +0.117 | -0.706 | +0.025 | +0.146 | -0.682 |
| 12 | +0.214 | -0.122 | -0.373 | +0.191 | -0.088 | -0.356 | +0.164 | -0.055 | -0.342 |
| 13 | +0.275 | -0.208 | -0.113 | +0.250 | -0.171 | -0.106 | +0.221 | -0.135 | -0.100 |
| 14 | +0.287 | -0.222 | 0 | +0.261 | -0.184 | 0 | +0.232 | -0.148 | 0 |


SINGLE BARREL CONDUIT BEGGS DEFORMETER STRESS ANALYSIS COEFFICIENTS FOR MOMENT, THRUST, AND SHEAR
TRIANGULAR VERTIGAL LOAD - TRIANGULAR FOUNDATION REACTION SHAPES $D, E, A N D F$

wore: 9 represents the weight per unit
volume of soll cover on the arch of
with those of the radius $r$

SINGLE BARREL CONDUIT BEGGS DEFORMETER STRESS ANALYSIS COEFFICIENTS FOR MOMENT, THRUST, AND SHEAR vertical arch load - uniform foundation reaction
SHAPES D, E, AND F

Note: No vertical orch lood on Shope $D$.
w


| POINT | $\frac{M}{9 r^{3}}$ | $\frac{T}{g^{2}}$ | $\frac{S}{9 r^{2}}$ | $\frac{M}{4.3}$ | $\frac{T}{9 r^{2}}$ | $\frac{S}{9 r^{2}}$ | $\frac{\mathrm{M}}{\mathrm{gr}}$ | $\frac{\mathrm{T}}{\mathrm{g} \mathrm{r}^{2}}$ | $\frac{\mathrm{S}}{\mathrm{gr}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +0.0.28 | 0.014 | 0 | +0.029 | -0.009 | 0 | +0.025 | -0.005 | 0 |
| 2 | +0.027 | -0.012 | +0.010 | +0.028 | -0.007 | +0.007 | +0.025 | -0.004 | +0.0 |
| 3 | +0.02 | +0.012 | +0.049 | +0.024 | +0.011 | +0.038 | +0.022 | +0.010 | +0.02 |
| 4 | +0.011 | +0.092 | +0.113 | +0.013 | +0.075 | +0.087 | +0.011 | $+0.058$ | +0 |
| 5 | 0.0 | +0. | +0 | -0.005 | +0.189 | +0.120 | -0.006 | +0.146 | +0 |
| 6 | -0.023 | +0.402 | $+0.122$ | -0.023 | +0.318 | +0.094 | -0.024 | +0.24 | +0.07 |
| 7 | -0.031 | +0.483 | +0.014 | -0.032 | +0.382 | +0.009 | -0.0 | +0.292 | +0 |
| 8 | -0.035 | +0.483 | +0.014 | -0.035 | +0.382 | +0.009 | -0.034 | +0.292 | +0. |
| 9 | -0.040 | +0.483 | +0.014 | -0.038 | +0.382 | +0.009 | -0.03 | 29 | +0.00 |
| 10 | -0.045 | +0.483 | +0.014 | -0.041 | +0.382 | +0.009 | -0.037 | +0.292 | +0.00 |
| 11 | +0.001 | +0.014 | -0.215 | -0.008 | +0.009 | -0.215 | -0.020 | +0.005 | -0.21 |
| 12 | +0.051 | +0.014 | -0.095 | +0.042 | +0.009 | -0.095 | +0.030 | +0.005 | -0.09 |
| 13 | +0.070 | +0.014 | -0.024 | +0.061 | +0.009 | -0.024 | +0.049 | +0.005 | -0.02 |
| 14 | +0.07 | +0.0 | 0 | +0.064 | +0.00 | 0 | $+0.05$ | 0.0 |  |

SINGLE BARREL CONDUIT BEGGS DEFORMETER STRESS ANALYSIS COEFFICIENTS FOR MOMENT, THRUST, AND SHEAR VERTICAL ARCH LOAD-TRIANGULAR FOUNDATION REACTION SHAPES D, E, AND F
NOTE: $g$ represents the weight per unit
volume of soil cover on the arch of
the conduit section in units consistent
with thase of the rodius $r$


+ Sign convention

| POINT | $\frac{\mathrm{M}}{9 \mathrm{r}^{3}}$ | $\frac{\mathrm{T}}{\mathrm{gr}}$ | $\frac{\mathrm{S}}{\mathrm{gr}}{ }^{2}$ | $\frac{\mathrm{M}}{\mathrm{gr}}$ | $\frac{\mathrm{T}}{\mathrm{gr}}$ | $\frac{5}{9 r^{2}}$ | $\frac{M}{9 r^{3}}$ | $\frac{\mathrm{T}}{\mathrm{gr}}$ | $\frac{S}{9 r^{2}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | +0.036 | -0.026 | 0 | +0.029 | -0.019 | 0 | +0.026 | -0.01 | 0 |
| 2 | +0.034 | -0.023 | +0.013 | +0.028 | -0.017 | +0.010 | +0.025 | 0.0 | +0.0 |
| 3 | +0.029 | +0.002 | +0.055 | +0.023 | +0.003 | +0.043 | +0.02 | +0.004 | +0.032 |
| 4 | +0.015 | +0.084 | +0.12 | +0.010 | +0.068 | +0.094 | +0.010 | +0.05 | +0.071 |
| 5 | -0. | +0. | +0.164 | -0.010 | +0.184 | +0.128 | -0.009 | +0.142 | +0.0 |
| 6 | -0.025 | +0.399 | +0.133 | -0.031 | +0.315 | $+0.104$ | -0.029 | $+0.242$ | +0. |
| 7 | -0.037 | +0.483 | +0.0 | -0.043 | +0.382 | +0.019 | -0.039 | +0.292 | +0.01 |
| 8 | -0.039 | +0.466 | -0.040 | -0.042 | +0.367 | -0.033 | -0.038 | +0.280 | -0.02 |
| 9 | -0.028 | +0.412 | -0.093 | -0.032 | +0.322 | . 07 | -0.027 | +0.244 | -0.058 |
| 10 | -0.010 | +0.330 | -0.12 | -0.014 | +0.256 | -0.095 | -0.011 | +0.191 | -0.073 |
| 11 | +0.002 | +0.117 | -0.196 | -0.005 | +0.098 | -0.173 | -0.006 | +0.08 | -0.150 |
| 12 | +0.037 | +0.054 | -0.093 | +0.028 | +0.043 | -0.082 | +0.024 | +0.034 | -0.072 |
| 13 | +0.050 | +0.029 | -0.023 | +0.040 | +0.022 | -0.020 | +0.035 | +0.015 | 0.01 |
| 14 | +0.05 | +0.026 | 0 | $+0.041$ | +0.019 | 0 | $+0.037$ | +0.01 | 0 |

4






SINGLE BARREL CONDUIT BEGGS DEFORMETER STRESS ANALYSIS COEFFICIENTS FOR MOMENT, THRUST, AND SHEAR

TRIANGULAR HORIZONTAL LOAD - BOTH SIDES
SHAPES $D, E$, AND $F$



NOTE: w represents the weight per unit
volume of water in units consistent
volume of woter in units consistent
with those of the radius $r$
with those of the rodius $r$.


SINGLE BARREL CONDUIT BEGGS DEFORMETER STRESS ANALYSIS COEFFICIENTS FOR MOMENT, THRUST, AND SHEAR TRIANGULAR INTERNAL RADIAL LOAD - UNIFORM FOUNDATION REACTION SHAPES $D, E, A N D ~ F$


FIGURE 33



FIGURE 35



| POINT | $\frac{M}{v r^{2}}$ | $\frac{T}{\mathrm{vr}}$ | $\frac{s}{v r}$ | $\frac{\mathrm{m}}{\mathrm{v} \mathrm{r}^{\mathbf{2}}}$ | $\frac{\mathrm{T}}{\mathrm{v}}$ | $\frac{s}{v r}$ | $\frac{\mathrm{M}}{\mathrm{vr}}$ | $\frac{T}{v i}$ | $\frac{s}{\mathrm{vr}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +1.077 | +0.217 | +1.500 | +0.903 | +0.178 | +1.333 | +0.735 | +0.142 | +1.167 |
| 2 | +0.601 | +0.598 | +1.393 | +0.508 | +0.517 | $+1.242$ | +0.413 | +0.439 | +1.090 |
| 3 | +0.176 | +0.938 | +1.190 | +0.154 | +0.821 | +1.065 | +0.123 | +0.706 | +0.939 |
| 4 | -0.170 | +1.214 | +0.907 | -0.135 | +1.069 | +0.817 | -0.114 | +0.926 | +0.724 |
| 5 | -0.411 | +1.408 | +0.562 | -0.339 | +1.244 | +0.512 | -0.283 | +1.081 | +0.460 |
| 6 | -0.533 | +1.505 | +0.178 | -0.444 | +1.334 | +0.173 | -0.372 | +1.164 | $+0.165$ |
| 7 | -0.527 | +1.500 | -0.217 | -0.443 | +1.333 | -0.178 | -0.375 | +1.167 | -0.142 |
| 8 | -0.406 | +1.343 | -0.585 | -0.345 | $+1.198$ | -0.506 | -0.296 | +1.052 | -0.429 |
| 9 | -0.204 | +1.016 | -0.838 | -0.173 | +0.911 | -0.732 | -0.152 | +0.804 | -0.628 |
| 10 | +0.040 | +0.596 | -0.904 | +0.037 | +0.540 | -0.793 | +0.025 | +0.483 | -0.684 |
| 11 | +0.271 | +0.187 | -0.758 | +0.237 | +0.179 | -0.667 | +0.196 | +0.169 | -0.576 |
| 12 | +0.435 | -0.109 | -0.431 | +0.380 | -0.083 | -0.380 | +0.318 | -0.059 | -0.328 |
| 13 | +0.495 | -0.217 | 0 | +0.431 | -0.178 | 0 | +0.362 | -0.142 | 0.32 |



| POINT | $\frac{M}{v r^{2}}$ | $\frac{T}{v r}$ | $\frac{s}{v r}$ | $\frac{\mathrm{M}}{\mathrm{v} \mathrm{r}^{\text {i }}}$ | $\frac{T}{v r}$ | $\frac{s}{v r}$ | $\frac{M}{y r^{2}}$ | $\frac{T}{v r}$ | $\frac{5}{\mathrm{~V}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +1.321 | +0.142 | +1.500 | +1.102 | +0.113 | +1.333 | +0.910 | +0.086 | I. |
| 2 | +0.571 | +0.142 | $+1.500$ | +0.435 | +0.113 | +1.333 | +0.327 | +0.086 | +1.187 |
| 3 | -0.179 | +0.142 | $+1.500$ | -0.232 | $+0.113$ | $+1.333$ | -0.257 | +0.086 | +1.16 |
| 4 | -0.518 | +1.500 | -0.142 | -0.435 | +1.333 | -0.113 | -0.347 | +1.167 | -0.086 |
| 5 | -0.447 | +1.500 | -0.142 | -0.378 | +1.333 | -0.113 | -0.304 | +1.167 | -0.086 |
| 6 | -0.376 | +1.500 | -0.142 | -0.322 | +1.333 | -0.113 | -0.261 | $+1.167$ | -0.086 |
| 7 | -0.305 | +1.500 | -0.142 | -0.265 | +1.333 | -0.113 | -0.218 | +1.167 | -0.086 |
| 8 | -0.234 | $+1.500$ | -0.142 | -0.208 | +1.333 | -0.113 | -0.176 | +1.167 | -0.086 |
| 9 | +0.052 | -0.142 | -1.000 | -0.023 | -0.113 | -1.000 | -0.085 | -0.086 | -1.000 |
| 10 | +0.487 | -0.142 | -0.500 | +0.352 | -0.113 | -0.500 | +0.290 | -0.086 | -0.50 |
| 11 | +0.53 | -0.142 | 0 | +0.477 | -0.113 | 0 | +0.415 | -0.086 | 0 |



| POINT | $\frac{\mathrm{M}}{\mathrm{VF}}$ | $\frac{T}{v T}$ | $\frac{\mathrm{s}}{\mathrm{vr}}$ | $\frac{\mathrm{M}}{\mathrm{v}} \mathrm{r}^{\mathbf{2}}$ | $\frac{\mathrm{T}}{\mathrm{vr}}$ | $\frac{\mathrm{S}}{\mathrm{Vr}}$ | $\frac{\mathrm{M}}{\mathrm{Vr}}$ | $\frac{\mathrm{T}}{\mathrm{vr}}$ | s |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +1.069 | +0.234 | +1.500 | +0.900 | +0.200 | +1.333 | +0.734 | +0.160 | +1.167 |
| 2 | +0.593 | +0.614 | $+1.388$ | +0.505 | +0.538 | $+1.236$ | +0.413 | +0.457 | +1.085 |
| 3 | +0.170 | +0.953 | +1.182 | +0.153 | +0.839 | +1.055 | +0.126 | +0.722 | +0.930 |
| 4 | -0.172 | +1.226 | +0.895 | -0.132 | +1.084 | +0.802 | -0.109 | +0.938 | +0.711 |
| 5 | -0.409 | +1.416 | +0.547 | -0.331 | $+1.254$ | $+0.494$ | -0.273 | +1.091 | +0.444 |
| 6 | -0.526 | +1.509 | +0.162 | -0.430 | +1.340 | +0.152 | -0.358 | +1.168 | 40.147 |
| 7 | -0.514 | +1.500 | -0.234 | -0.423 | +1.333 | -0.200 | -0.356 | $+1.167$ | -0.160 |
| 8 | -0.456 | +1.500 | -0.234 | -0.373 | +1.333 | -0.200 | -0.316 | +1.167 | -0.160 |
| 9 | -0.397 | +1.500 | -0.234 | -0.323 | +1.333 | -0.200 | -0.276 | +1.167 | -0.160 |
| 10 | -0.315 | +1.444 | -0.467 | -0.255 | +1.282 | -0.417 | -0.221 | +1.120 | -0.363 |
| 11 | -0.180 | +1.353 | -0.689 | -0.139 | +1.195 | -0.624 | -0.124 | +1.040 | -0.553 |
| 12 | +0.057 | +0.109 | -1.085 | +0.027 | +0.144 | -1.029 | -0.024 | +0.184 | -0.965 |
| 13 | +0.257 | -0.109 | -0.802 | $+0.213$ | -0.072 | -0.775 | +0.147 | -0.031 | -0.744 |
| 14 | +0.403 | -0.234 | -0.500 | +0.351 | -0.200 | -0.500 | +0.276 | -0.160 | -0.500 |
| 15 | +0.497 | -0.234 | -0.250 | +0.445 | -0.200 | -0.250 | +0.370 | -0.160 | -0.250 |
| 16 | +0.528 | -0.234 | 0 | +0.476 | -0.200 | 0 | +0.401 | -0.160 | 0 |

SINGLE BARREL CONDUIT BEGGS DEFORMETER STRESS ANALYSIS COEFFICIENTS FOR MOMENT, THRUST, AND SHEAR CONCENTRATED VERTIGAL LOAD - UNIFORM FOUNDATION REACTION SHAPES CIRCULAR, SQUARE, AND $O$


| POINT | $\frac{M}{v r^{2}}$ | $\frac{\mathrm{T}}{\mathrm{vr}}$ | $\frac{5}{v r}$ | $\frac{M}{v r^{2}}$ | $\frac{\mathrm{T}}{\mathrm{va}}$ | $\frac{5}{v r}$ | $\frac{\mathrm{m}}{\mathrm{v} \mathrm{r}^{\text {b }}}$ | $\frac{T}{v r}$ | $\frac{s}{v r}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +1.025 | +0.327 | +1.500 | +0.859 | +0.269 | +1.333 | +0.702 | +0.214 | $+1.167$ |
| 2 | +0.554 | $+0.704$ | $+1.364$ | +0.467 | +0.605 | +1.218 | +0.383 | +0.308 | $+1.072$ |
| 3 | +0.143 | +1.033 | $+1.136$ | +0.123 | +0.899 | $+1.020$ | +0.101 | +0.768 | +0.903 |
| 4 | -0.181 | +1.292 | +0.829 | -0.149 | +1.133 | +0.753 | -0.124 | +0.976 | +0.674 |
| 5 | -0.394 | +1.463 | +0.467 | -0.331 | +1.289 | +0.434 | -0.277 | +1.117 | +0.398 |
| 6 | -0.463 | +1.534 | +0.072 | -0.412 | +1.357 | +0.086 | -0.347 | $+1.182$ | +0.095 |
| 7 | -0.441 | $+1.500$ | -0.327 | -0.383 | +1.333 | -0.269 | -0.330 | +1.167 | -0.214 |
| 8 | -0.298 | +1.267 | -0.678 | -0.265 | +1.132 | -0.581 | -0.235 | +0.996 | -0.488 |
| 9 | -0.105 | +0.811 | -0.846 | -0.097 | +0.732 | -0.733 | -0.090 | +0.651 | -0.623 |
| 10 | +0.091 | +0.299 | -0.762 | +0.078 | +0.281 | -0.661 | +0.064 | +0.261 | -0.564 |
| 11 | +0.241 | -0.096 | -0.488 | +0.212 | -0.066 | -0.423 | +0.182 | -0.039 | -0.359 |
| 12 | +0.322 | -0.290 | -0.182 | +0.284 | -0.236 | -0.156 | +0.244 | -0.186 | -0.13 |
| 13 | +0.343 | -0.327 | [ 0 | +0.301 | -0.269 | 0 | +0.258 | -0.214 | 0 |


SINGLE BARREL CONDUIT BEGGS DEFORMETER STRESS ANALYSIS COEFFICIENTS FOR MOMENT, THRUST, AND SHEAR concentrated vertical load - triangular foundation reaction SHAPES GIRCULAR, SQUARE, AND G




| POIN T | $\frac{\mathrm{M}}{\underline{g r}{ }^{3}}$ | $\frac{T}{g r^{2}}$ | $\frac{S}{g r^{2}}$ | $\frac{M}{g r^{3}}$ | $\frac{T}{g r^{2}}$ | $\frac{S}{g r^{2}}$ | $\frac{M}{9 r^{3}}$ | $\frac{\mathrm{T}}{\mathrm{gr}}$ | $\frac{\mathrm{S}}{\underline{0} \mathrm{r}^{2}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +0.051 | -0.066 | 0 | +0.043 | -0.048 | 0 | +0.035 | -0.033 | \% |
| 2 | +0.048 | -0.062 | +0.023 | $+0.041$ | -0.045 | +0.017 | +0.033 | -0.031 | +0.012 |
| 3 | +0.037 | -0.032 | +0.075 | +0.033 | -0.022 | +0.057 | +0.087 | -0.014 | +0.042 |
| 4 | +0.016 | +0.056 | +0.149 | +0.014 | +0.047 | +0.115 | +0.012 | +0.039 | +0.085 |
| 5 | -0.015 | +0.213 | +0.199 | -0.013 | +0.170 | +0.153 | -0.012 | +0.132 | $+0.114$ |
| 6 | -0.047 | +0.388 | +0.172 | -0.042 | +0.308 | +0.132 | -0.037 | +0.237 | +0.098 |
| 7 | -0.072 | +0.483 | +0.066 | -0.063 | +0.382 | +0.048 | -0.053 | +0.292 | +0.033 |
| 8 | -0.077 | +0.468 | -0.057 | -0.064 | +0.368 | -0.049 | -0.052 | +0.281 | -0.041 |
| 9 | -0.053 | +0.395 | -0.152 | -0.043 | +0.310 | -0.124 | -0.034 | +0.236 | -0.098 |
| 10 | -0.009 | +0.288 | -0.195 | -0.007 | +0.225 | -0.157 | -0.005 | +0.160 | -0.123 |
| 11 | +0.038 | +0.178 | -0.176 | +0.032 | +0.137 | -0.141 | +0.026 | +0.102 | -0.110 |
| 12 | +0.074 | +0.096 | -0.104 | +0.062 | +0.072 | -0.083 | +0.049 | +0.052 | -0.064 |
| 13 | +0.087 | +0.066 | 0 | +0.072 | +0.048 | 0 | +0.057 | +0.033 | 0 |

Note: No vertical orch lood on square shope.


NOTE: 9 represents the weight per unit volume of soil cover on the arch of the conduit section in units consistent with those of the radius $r$.

| POINT | $\frac{\mathrm{m}}{9 \mathrm{r}^{3}}$ | $\frac{\mathrm{T}}{9 r^{2}}$ | $\frac{S}{\rho r^{2}}$ | $\frac{\mathrm{M}}{9 \mathrm{r}^{3}}$ | $\frac{1}{g r^{2}}$ | $\frac{5}{0 r^{2}}$ | $\frac{M}{g r^{3}}$ | $\frac{T}{9 r^{2}}$ | $\frac{3}{g r^{2}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +0.048 | -0.053 | 0 | +0.042 | -0.038 | 0 | +0.035 | 0.027 | 0 |
| 2 | +0.045 | -0.049 | +0.020 | +0.040 | -0.036 | +0.015 | +0.033 | -0.025 | +0.01 |
| 3 | +0.036 | -0.02 | +0.069 | +0.033 | -0.014 | +0.053 | +0.028 | -0.008 | +0.039 |
| 4 | +0.017 | +0.06 | $+0.140$ | +0.016 | +0.054 | +0.108 | +0.014 | +0.043 | +008 |
| 5 | 0.011 | +0.219 | +0.188 | -0.009 | +0.175 | +0.145 | -0.008 | +0.135 | +0.109 |
| 6 | -0.039 | +0.391 | +0.160 | -0.036 | +0.310 | +0.1 | -0.032 | +0.238 | +0. |
| 7 | -0.060 | +0.483 | +0.053 | -0.053 | +0.382 | +0.038 | -0.046 | +0.292 | 27 |
| 0 | . 07 | +0.483 | +0.053 | -0.063 | +0.382 | $+0.038$ | -0.053 | +0.292 | +0.027 |
| 9 | -0.086 | +0.483 | +0.053 | -0.0 | +0.382 | +0.038 | -0.059 | +0.292 | +0.027 |
| 10 | 0.090 | +0. | 0 | -0.07 | +0.3 | -0.025 | -0.059 | +0.292 | . 0 |
| 11 | 0.075 | +0 | -0.100 | -0.06 | +0.3 | -0.088 | -0.049 | +0.283 | -0.0 |
| 12 | 0.02 | +0.157 | -0.309 | -0.029 | +0. 1 | -0.263 | -0.030 | +0.109 | -0.218 |
| 13 | +0.030 | +0.092 | -0.238 | +0.020 | +0.07 | -0.206 | +0.009 | +0.058 | 0.175 |
| 14 | +0.075 | +0.053 | -0.161 | +0.057 | +0.038 | -0.143 | +0.040 | +0.027 | -0.125 |
| 15 | +0.106 | +0.053 | -0.080 | +0.084 | +0.038 | -0.072 | +0.063 | +0.027 | -0.063 |
| 16 | +0.116 | +0.053 | 0 | +0.093 | +0.038 | 0 | +0.071 | +0.027 | 0 |



SINGLE BARREL CONDUIT BEGGS DEFORMETER STRESS ANALYSIS COEFFICIENTS FOR MOMENT, THRUST, AND SHEAR VERTICAL ARCH LOAO - UNIFORM FOUNDATION REACTION SHAPES CIRCULAR, SQUARE, AND G


| POINT | $\frac{M}{9 r^{3}}$ | $\frac{T}{9 r^{2}}$ | $\frac{5}{\underline{9} r^{2}}$ | $\frac{m}{0 r^{3}}$ | $\frac{T}{g r^{2}}$ | $\frac{3}{\text { gr }}$ | $\frac{\mathrm{M}}{\mathrm{gr}}$ | $\frac{\mathrm{T}}{\mathrm{g} \mathrm{r}^{2}}$ | $\frac{\mathrm{S}}{\mathbf{g r}{ }^{\mathbf{2}}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +0.035 | -0.030 | 0 | +0.031 | -0.022 | 0 | +0.027 | -0.015 | 0 |
| 2 | +0.033 | -0.028 | +0.014 | +0.029 | -0.020 | +0.011 | +0.026 | -0.014 | +0.008 |
| 3 | +0.027 | -0.002 | +0.057 | +0.024 | +0.000 | +0.044 | +0.021 | +0.002 | +0.033 |
| 4 | +0.012 | +0.081 | +0.124 | +0.010 | +0.065 | +0.097 | +0.009 | +0.051 | +0.073 |
| 5 | -0.010 | +0.230 | +0.168 | -0.011 | +0.183 | +0.131 | -0.010 | $+0.141$ | +0.099 |
| 6 | -0.031 | +0.397 | +0.138 | -0.033 | +0.314 | +0.107 | -0.030 | +0.241 | +0.080 |
| 7 | -0.044 | +0.483 | +0.030 | -0.045 | +0.382 | +0.022 | -0.042 | +0.292 | +0.015 |
| 8 | -0.042 | +0.443 | -0.087 | -0.041 | +0.350 | -0.071 | -0.037 | +0.267 | -0.056 |
| 9 | -0.021 | +0.329 | -0.155 | -0.021 | +0.259 | -0.124 | -0.019 | +0.197 | -0.096 |
| 10 | +0.007 | +0. 192 | -0.149 | +0.005 | +0.150 | -0.119 | +0.004 | +0.114 | -0.093 |
| 11 | +0.028 | +0.087 | -0.089 | +0.025 | +0.067 | -0.072 | +0.022 | +0.050 | -0.056 |
| 12 | +0.038 | +0.038 | -0.023 | +0.034 | +0.028 | -0.019 | +0.030 | +0.020 | -0.015 |
| 13 | +0.038 | +0.030 | 0 | +0.035 | +0.022 | 0 | +0.031 | +0.015 | 0 |

Note: No vertical orch lood on squore shope.


| POINT | $\frac{\mathrm{M}}{9 r^{3}}$ | $\frac{T}{\text { gre }}$ | $\frac{5}{9 r^{2}}$ | $\frac{\mathrm{M}}{9 \mathrm{r}^{3}}$ | $\frac{T}{9 r^{2}}$ | $\frac{5}{9 r^{2}}$ | $\frac{\mathrm{m}}{9}$ | $\frac{T}{\text { Tre }}$ | $\frac{s}{9 r}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +0.033 | -0.023 | 0 | +0.031 | -0.016 | 0 | +0.027 | -0.010 | 0 |
| 2 | +0.032 | -0.021 | +0.012 | +0.030 | -0.014 | +0.009 | +0.026 | -0.009 | +0.007 |
| 3 | +0.026 | $+0.004$ | +0.054 | +0.025 | +0.006 | +0.041 | +0.022 | +0.006 | +0.031 |
| 4 | +0.013 | +0.086 | +0.119 | +0.013 | +0.070 | +0.092 | +0.011 | +0.055 | +0.069 |
| 5 | -0.007 | +0.234 | +0.162 | -0.007 | +0.186 | +0.128 | -0.007 | +0.143 | +0.095 |
| 6 | -0.026 | +0.399 | +0.131 | -0.027 | +0.316 | $+0.101$ | -0.028 | +0.242 | +0.076 |
| 7 | -0.037 | +0.483 | +0.023 | -0.038 | +0.382 | +0.016 | -0.036 | +0.292 | -0.0 |
| 8 | -0.043 | +0.483 | $+0.023$ | -0.042 | +0.382 | +0.016 | -0.039 | +0.292 | +0.010 |
| 9 | -0.048 | +0.483 | +0.023 | -0.046 | +0.382 | +0.016 | -0.042 | +0.292 | +0.010 |
| 10 | -0.045 | +0.480 | -0.053 | -0.042 | +0.379 | -0.047 | -0.038 | +0.289 | -0.041 |
| 11 | -0.024 | +0.466 | -0.128 | -0.025 | +0.366 | -0.110 | -0.024 | +0.278 | -0.091 |
| 12 | -0.004 | +0.098 | -0.224 | -0.006 | +0.088 | -0.806 | -0.011 | +0.079 | -0.185 |
| 13 | +0.028 | +0.043 | -0.124 | +0.023 | +0.036 | -0.117 | +0.016 | +0.030 | -0.110 |
| 14 | +0.046 | +0.023 | -0.054 | +0.041 | +0.016 | -0.054 | +0.033 | +0.010 | -0.054 |
| 15 | +0.054 | +0.023 | -0.013 | +0.049 | +0.018 | -0.013 | +0.040 | +0.010 | 0.013 |
| 16 | +0.055 | +0.023 | 0 | +0.050 | +0.016 | 0 | +0.042 | +0.010 | 0 |

AEV. APR. 15, 1988

Nove: $g$ represente the waight par unil volume of soil cover on the orch of the conduit section in units consistent with those of the radius $r$

## SINGLE BARREL CONDUIT BEGGS DEFORMETER STRESS ANALYSIS

 COEFFICIENTS FOR MOMENT, THRUST, AND SHEAR VERTICAL ARCH LOAD - TRIANOULAR FOUNDATION REACTION SHAPES GIRCULAR, SQUARE, AND E







| POINT | $\frac{M}{w r^{3}}$ | $\frac{T}{\text { w } r^{\text {e }}}$ | $\frac{\mathrm{s}}{\mathbf{w r}}{ }^{\text {e }}$ | $\frac{M}{w r^{3}}$ | $\frac{T}{\text { wr }{ }^{\text {e }}}$ | $\frac{S}{w r^{2}}$ | $\frac{M}{w r^{3}}$ | $\frac{\mathrm{T}}{\mathrm{wr} \mathrm{r}^{2}}$ | $\frac{s}{w r^{2}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | +0.179 | -0.605 | 0 | +0.171 | -0.619 | 0 | +0.158 | -0.629 | 0 |
| 2 | +0.179 | -0.605 | 0 | +0.171 | -0.619 | 0 | +0.158 | -0.629 | 0 |
| 3 | +0.179 | -0.605 | 0 | +0.171 | -0.619 | 0 | +0.158 | -0.629 | 0 |
| 4 | +0.028 | 0 | +0.605 | +0.068 | 0 | +0.619 | +0.106 | 0 | +0.629 |
| 5 | -0.254 | 0 | +0.480 | -0.220 | 0 | +0.494 | -0.188 | 0 | +0.504 |
| 6 | -0.41 | 0 | +0.105 | -0.384 | 0 | +0.119 | -0.356 | 0 | +0.129 |
| 7 | -0.317 | 0 | -0.520 | -0.297 | 0 | -0.506 | -0.275 | 0 | -0.496 |
| 8 | +0.151 | 0 | -1.395 | +0.164 | 0 | -1.38 | +0.181 | 0 | -1.371 |
| 9 | +0.334 | -1.395 | +0.667 | +0.311 | -1.381 | +0.500 | +0.272 | -1.371 | +0.286 |
| 10 | +0.084 | -1.395 | +0.333 | +0.123 | -1.381 | +0.250 | +0.165 | -1.371 | +0.143 |
| 11 | +0.000 | -1.395 | 0 | +0.06 1 | -1.301 | 0 | +0.129 | -1.371 | 0 |


SINGLE BARREL CONDUIT BEGGS DEFORMETER STRESS ANALYSIS
COEFFICIENT'S FOR MOMENT, THRUST, AND SHEAR TRIANGULAR INTERNAL RADIAL LOAD - UNIFORM FOUNDATION REACTION SHAPES GIRCULAR, SQUARE, AND G



SHAPE A

| SHAPE A |  |  |  |
| :---: | :---: | :---: | :---: |
| POINT | $\frac{\mathrm{m}}{\mathbf{w} \mathrm{r}^{3}}$ | $\frac{T}{w r^{2}}$ | $\frac{S}{w r^{2}}$ |
| 1 | +0.057 | +0.728 | 0 |
| 2 | +0.049 | +0.772 | +0.069 |
| 3 | +0.027 | +0.892 | +0.114 |
| 4 | -0.001 | +1.065 | +0.117 |
| 5 | -0.024 | +1.253 | 40.079 |
| 6 | -0.032 | +1.423 | +0.014 |
| 7 | -0.025 | +1.549 | -0.048 |
| 8 | -0.048 | +1.689 | +0.224 |
| 9 | -0.154 | +1.913 | +0.561 |
| 10 | -0.356 | +2.266 | +0.954 |
| 11 | -0.323 | +2.281 | -1.172 |
| 12 | -0.072 | +2.127 | -0.829 |
| 13 | +0.123 | +2.028 | -0.429 |
| 14 | +0.098 | +1.994 | 0 |

SHAPE E
SHAPE E

| POINT | $\frac{M}{w r^{3}}$ | $\frac{T}{w r^{2}}$ | $\frac{S}{w r^{2}}$ |
| :---: | :---: | :---: | :---: |
| 1 | +0.061 | +0.907 | 0 |
| 2 | +0.049 | +0.964 | +0.099 |
| 3 | +0.018 | +1.123 | +0.156 |
| 4 | $-0.017+1.343$ | +0.143 |  |
| 5 | $-0.038+1.570$ | +0.056 |  |
| 6 | $-0.028+1.753$ | -0.082 |  |
| 7 | $+0.019+1.854$ | -0.226 |  |
| 8 | $+0.023+1.988+0.218$ |  |  |
| 9 | $-0.139+2.121$ | +0.774 |  |
| 10 | $-0.505+2.255$ | +1.441 |  |
| 11 | $-0.464+1.816$ | -1.933 |  |
| 12 | $+0.073+1.816$ | -1.288 |  |
| 13 | $+0.395+1.616$ | -0.644 |  |
| 14 | $+0.503+1.816$ | 0 |  |

CIRCULAR
CIRCULAR

| POINT | $\frac{M}{w r^{3}}$ | $\frac{T}{w r^{2}}$ | $\frac{S}{w r^{2}}$ |
| :---: | :---: | :---: | :---: |
| 1 | +0.106 | +0.707 | 0 |
| 2 | +0.090 | +0.764 | +0.125 |
| 3 | +0.047 | +0.923 | +0.211 |
| 4 | -0.009 | +1.149 | +0.232 |
| 5 | $-0.062++1.391$ | +0.184 |  |
| 6 | -0.094 | +1.603 | +0.085 |
| 7 | -0.100 | +1.748 | -0.026 |
| 6 | -0.080 | +1.842 | -0.109 |
| 9 | -0.042 | +1.912 | -0.152 |
| 10 | +0.003 | +1.961 | -0.155 |
| 11 | +0.043 | +1.993 | -0.125 |
| 12 | +0.071 | +2.010 | -0.069 |
| 13 | +0.081 | +2.016 | 0 |

SHAPE $B$

| SHAPE ${ }^{\text {P }}$ |  |  |  |
| :---: | :---: | :---: | :---: |
| POINT | $\frac{M}{w r^{3}}$ | $\frac{T}{w r^{2}}$ | $\frac{S}{w r^{2}}$ |
| 1 | +0.019 | +0.679 | 0 |
| 2 | +0.015 | +0.710 | +0.029 |
| 3 | +0.007 | +0.797 | +0.047 |
| 4 | -0.003 | +0.924 | +0.046 |
| 5 | -0.010 | +1.070 | +0.030 |
| 6 | -0.012 | $+1.213$ | +0.009 |
| 7 | -0.012 | +1.338 | +0.002 |
| 8 | -0.035 | +1.489 | +0.192 |
| 9 | -0.106 | +1.770 | +0.421 |
| 10 | -0.216 | +2.256 | +0.660 |
| 11 | -0.194 | +2.452 | -0.696 |
| 12 | -0.067 | +2.238 | -0.537 |
| 13 | +0.031 | +2.094 | -0.291 |
| 14 | +0.067 | +2.043 | 0 |

SHAPE C

| POINT | $\frac{M}{w r^{3}}$ | $\frac{T}{w r^{2}}$ | $\frac{S}{w r^{2}}$ |
| :---: | :---: | :---: | :---: |
| 1 | +0.080 | +0.687 | 0 |
| 2 | +0.068 | +0.737 | +0.099 |
| 3 | +0.034 | +0.876 | +0.167 |
| 4 | -0.010 | +1.073 | +0.184 |
| 5 | -0.051 | +1.289 | +0.147 |
| 6 | -0.078 | +1.483 | +0.073 |
| 7 | -0.085 | +1.625 | -0.007 |
| 8 | -0.102 | +1.785 | +0.157 |
| 9 | -0.162 | +2.094 | +0.363 |
| 10 | -0.148 | +2.099 | -0.576 |
| 11 | -0.025 | +2.025 | -0.376 |
| 12 | +0.041 | +2.015 | -0.105 |
| 13 | +0.062 | +2.030 | -0.057 |
| 14 | +0.070 | +2.035 | 0 |

SHAPE 6
SHAPE G

| POINT | $\frac{M}{w r^{3}}$ | $\frac{T}{w r^{2}}$ | $\frac{S}{w r^{2}}$ |
| :---: | :---: | :---: | :---: |
| 1 | +0.083 | +0.778 | 0 |
| 2 | +0.07 | +0.832 | +0.100 |
| 3 | +0.036 | +0.980 | +0.165 |
| 4 | -0.003 | +1.188 | +0.170 |
| 5 | -0.037 | +1.410 | +0.112 |
| 6 | -0.049 | +1.599 | +0.010 |
| 7 | -0.033 | +1.725 | -0.090 |
| 8 | -0.048 | +1.825 | +0.225 |
| 9 | -0.151 | +1.925 | +0.611 |
| 10 | -0.279 | +2.163 | +0.645 |
| 11 | -0.400 | +2.535 | +0.667 |
| 12 | -0.364 | +2.450 | -1.052 |
| 13 | -0.163 | +2.160 | -1.042 |
| 14 | +0.045 | +1.944 | -0.968 |
| 15 | +0.286 | +1.944 | -0.483 |
| 16 | +0.206 | +1.944 | 0 |

Top reaction is ossumed to be of uniform infensity, $v$.

| SHAPE | $\frac{V}{w r}$ |
| :---: | :---: |
| A | +0.493 |
| $B$ | +0.312 |
| $C$ | +0.558 |
| $E$ | +0.754 |
| $F$ | +0.713 |
| G | +0.644 |
| Circular | +0.664 |
| Square | +0.845 |

For looding diogrom, + sign convention, and the assumption hat the conduits do not floot see Figures 18, 33, and 46.

Note: Shape D does not floot.

SINGLE BARREL CONDUIT BEGGS DEFORMETER STRESS ANALYSIS COEFFICIENTS FOR MOMENT, THRUST, AND SHEAR TRIANGULAR EXTERNAL HYDROSTATIC LOAD INCLUDING DEAD LOAD
CONDUITS ASSUMED TO FLOAT
ALL SHAPES
$t=\frac{r}{6}$

| POINT | $\frac{M}{u r}$ | $\frac{T}{u r}$ | $\frac{s}{u r}$ | $\frac{m}{u r^{2}}$ | $\frac{T}{u r}$ | $\frac{s}{u r}$ | $\frac{M}{u T}$ | $\frac{T}{u r}$ | $\frac{s}{u r}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | -0.218 | +0.530 | 0 | -0.183 | +0.471 | 0 | -0.150 | +0.412 | 0 |
| 2 | -0.196 | +0.512 | $-0.137$ | -0.164 | +0.455 | $-0.122$ | -0.135 | +0.398 | -0.107 |
| 3 | $-0.130$ | +0.459 | -0.265 | -0.109 | +0.408 | -0.236 | -0.090 | +0.357 | -0.206 |
| 4 | -0.024 | +0.375 | -0.375 | -0.021 | +0.333 | -0.333 | -0.019 | + 0.292 | -0.292 |
| 5 | +0.103 | +0.242 | -0.420 | +0.085 | $+0.215$ | $-0.373$ | +0.069 | +0.189 | -0.327 |
| 6 | +0.211 | +0.082 | $-0.3 .06$ | +0.179 | +0.073 | -0.272 | +0.149 | +0.064 | -0.238 |
| 7 | +0.257 | 0 | 0 | +0.219 | 0 | 0 | +0.183 | 0 | 0 |
| 8 | $+0.211$ | +0.082 | +0.306 | $+0.179$ | $+0.073$ | +0.272 | +0.149 | +0.064 | +0.238 |
| 9 | +0.103 | +0.242 | +0.420 | +0.085 | $+0.215$ | +0.373 | +0.069 | +0.189 | +0.327 |
| 10 | -0.024 | +0.375 | +0.375 | -. 0.021 | +0.333 | +0.333 | -0.019 | +0.292 | +0.292 |
| 11 | -0.130 | +0.459 | +0.265 | -0.109 | $+0.408$ | +0.236 | -0.090 | +0.357 | +0.206 |
| 12 | -0.196 | +0.512 | +0.137 | -0.164 | +0.455 | +0.122 | -0.135 | +0.398 | $+0.107$ |
| 13 | -0.218 | +0.530 | 0 | -0.183 | +0.471 | 0 | -0.150 | +0.412 | 0 |



+ Sign convention


SINGLE BARREL CONDUIT BEGGS DEFORMETER STRESS ANALYSIS COEFFICIENTS FOR MOMENT, THRUST, AND SHEAR

HORIZONTAL PASSIVE PRESSURE
CIRCULAR SHAPE

## APPENDIX: THE BEGGS DEFORMETER

This study has been made using the Beggs Deformeter apparatus ${ }^{466}$ (figure 51). The basis of the method is a direct application of Maxwell's Theorem of Reciprocal Deflections, which states that for any two points on a structure, the ratio of the displacement at the first point to the load causing it, applied at the second point, is equal to the ratio of the displacement at the second point to the load causing it, applied at the first point. Displacements are measured in the load directions.
In the general application of this method of stress analysis, an elastic scale model of the structure under consideration is deformed at a cut in the model by use of a special set of gage blocks and plugs. Three sets of plugs are used to apply a rotational, a normal, and a shearing displacement at the gage block. Microscopes equipped with filar eyepieces are used to measure the model deflections at points corresponding to the load points of the actual structure. Deflections are measured in the direction of the prototype loads. No loads are applied to the model. Deflections of the model are read at prototype load points for displacements applied at the gage block. The difference in microscope readings is a measure of the model deflection induced by the change at the gage block from the first position of the plugs to the second position of the plugs.
From Maxwell's Theorem the following equations may be written for the redundant reactions at the cut section:
$\left.\begin{array}{c}\begin{array}{c}\text { For a concentrated } \\ \text { load }\end{array} \\ M M_{1}=P \frac{e_{M}}{d_{M R}} n\end{array} \quad \begin{array}{c}\text { For a distributed } \\ \text { load }\end{array}\right]$

[^3]\[

$$
\begin{array}{ll}
S_{1}=P \frac{e_{S}}{d_{S}} & S_{1}=\frac{1}{d_{S}} \int p e_{S} d l \\
T_{1}=P \frac{e_{T}}{d_{T}} & T_{1}=\frac{1}{d_{T}} \int p e_{T} d l
\end{array}
$$
\]

where
$d_{M}$ is the angular rotation applied at the cut by the moment plugs
$d_{s}$ is the displacement applied at the cut by the shear plugs
$d_{T}$ is the displacement applied at the cut by the thrust plugs
$e_{M}$ is the measured deflection at a load point, in the direction of the load, due to $d_{M}$
$e_{S}$ is the measured deflection at a load point, in the direction of the load, due to $d_{s}$
$e_{r}$ is the measured deflection at a load point, in the direction of the load, due to $d_{T}$
$l$ is the load length
$M_{1}$ is the redundant moment reaction at the cut
$n$ is the scale factor (prototype to model)
$P$ is a load acting at a point on the prototype
$p$ is the load intensity on the prototype at the deflection point
$S_{1}$ is the redundant shear reaction at the cut
$T_{1}$ is the redundant thrust reaction at the cut.

The only unknowns in these equations are $M_{1}, T_{1}$, and $S_{1}$.

In the actual operation of the Beggs Deformeter the arithmetic is simplified by the use of calibration factors based on the plug dimensions and the eyepiece scales. An influence line through points obtained by multiplying the deflection ordinates by the proper calibration factor gives directly the magnitude of the moment, thrust, or shear at the gage block position for a unit traveling load.

It should be pointed out that the Beggs Deformeter method automatically takes into account the strain energy in a structure due to moment, thrust, and shear as well as haunch effects and other shape changes.


FIGURE 51. -Beggs Deformeter apparatus and shape B conduit model.

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[^0]:    ${ }^{2}$ Sandhu, R. S., "Design of Concrete Linings for Large Underground Conduits," Journal of the American Concrete Institute, December 1961, pp. 737-750.

[^1]:    ${ }^{3}$ Murphy, Glenn, Advanced Mechanics of Materials, McGraw-Hill Book Co., Inc., New York, 1946, pp. 217219.

[^2]:    REV. SEP. RB, 196

[^3]:    ${ }^{4}$ Beggs, G. E., "An Accurate Solution of Statically Indeterminate Structures by Paper Models and Special Gages," Proceedings ACI, vol. XVIII, 1922, pp. 58-78.
    ${ }^{5}$ McCullough, C. B., and Thayer, F. S., Elastic Arch Bridges, John Wiley and Sons, New York, 1931, pp. 282300.
    ${ }^{\circ}$ Phillips, H. B., and Allen, I. E., "The Beggs Deformeter Theory and Technique," Bureau of Reclamation, Denver, Colo., July 1965.

